



Stantec

Stantec Consulting Services Inc.
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February 15, 2012

ltr_002_175551015

Mr. Michael S. Turnbow
Tennessee Valley Authority
1101 Market Street, LP 2G-C
Chattanooga, Tennessee 37402-2801

Re: Results of Pseudostatic Slope Stability Analysis
Active CCP Disposal Facilities
BRF, COF, GAF, JSF, JOF, KIF, PAF, and WCF

Dear Mr. Turnbow:

As requested, Stantec Consulting Services Inc. (Stantec) has conducted pseudostatic slope stability analyses for ground motion levels corresponding to a return period of 2,500 years to support the U.S. Environmental Protection Agency's assessment of TVA's CCP disposal facilities. The results for Bull Run (BFR), Colbert (COF), Gallatin (GAF), John Sevier (JSF), Johnsonville (JOF), Kingston (KIF), Paradise (PAF), and Widows Creek (WCF) are provided in this letter.

Approach

The analyses were performed for current conditions using pseudostatic stability methods, where the added inertial load from an earthquake is assumed to be represented by a simple horizontal pseudostatic coefficient. Specifics related to the analyses/approach are as follows:

- Subsurface data was obtained from the Stantec's recent geotechnical studies performed in 2009 and 2010 time frame.
- SLOPE/W software (from GEO-SLOPE International, Inc.) was used to perform the calculations.
- One existing SLOPE/W cross-section model per disposal facility was selected from the previous studies for analysis. For simplicity and conservatism, the selected sections represent the facility's lowest current static (long-term) factor of safety. The SLOPE/W models were updated to reflect any significant mitigations or operational changes that have occurred since completion of Stantec's geotechnical studies.
- Undrained shear strength parameters were used.
- Ground motion levels corresponding to a return period of 2,500 years (or approximate exceedance probability of 2% in 50 years) was used for selection of a horizontal seismic coefficient. For simplicity, the horizontal seismic coefficient was selected to equal the total hazard peak ground acceleration (rock) for 2,500 year return periods as shown in plant-

Stantec Consulting Services Inc.
One Team. Infinite Solutions

specific tables (Tables 13 through 23) of TVA's March 28, 2011 region-specific seismic hazard study performed by AMEC Geomatrix, Inc.

- A target factor of safety (FS) of 1.0 was considered for comparing results.

Results

The results of the pseudostatic stability analyses are enclosed (summary spreadsheet, SLOPE/W cross-sections, and plan views showing cross-section locations). The results indicate factors of safety greater than or equal to the target of 1.0.

Stantec appreciates the opportunity to provide these services. If you have questions, or if we can provide additional information, please let us know.

Sincerely,

STANTEC CONSULTING SERVICES INC.

A handwritten signature in black ink that reads "Randy L. Roberts". The signature is written in a cursive, flowing style.

Randy L. Roberts, PE
Principal

Enclosures

/cdm

Pseudostatic Stability Analysis Summary - TVA Active CCP Disposal Facilities

BRF, COF, GAF, JSF, JOF, KIF, PAF, WCF

Plant	CCP Disposal Facility		Cross-Section	2,500 yr Return	
	Name	Type		PGA (g)	Factor of Safety
BRF	Gypsum Disposal Area 2A	Wet Stack	I	0.131	1.0
	Fly Ash Disposal Area 2	Impoundment	S		1.4
	Bottom Ash Disposal Area 1	Stack	D		1.1
COF	Disposal Area 5 Stack	Stack	I	0.138	1.0
	Disposal Area 5 Stilling Basin	Impoundment	J		1.2
	Ash Pond 4	Impoundment	D		1.0
GAF	Ash Pond A	Impoundment	K	0.108	1.0
	Ash Pond E	Impoundment	B		1.3
JSF	Bottom Ash Pond	Impoundment	I	0.115	2.2
JOF	Ash Disposal Area 2	Impoundment	K	0.254	1.0
KIF	Stilling Pond	Impoundment	132+37	0.115	1.0
PAF	Slag Ponds 2A and 2B	Impoundment	Typical	0.157	1.1
	Scrubber Sludge Complex	Impoundment	G		1.0
	Peabody Ash Pond	Impoundment	A		1.0
WCF	Gypsum Stack	Wet Stack	F	0.1	1.5
	Dredge Cell (Old Scrubber Sludge Pond)	Impoundment	D		1.1
	Main Ash Pond	Impoundment	J		1.4

Bull Run Fossil Plant (BRF)

**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**

**Section I - Gypsum Disposal Area 2A
Bull Run Fossil Plant
Clinton, Tennessee**

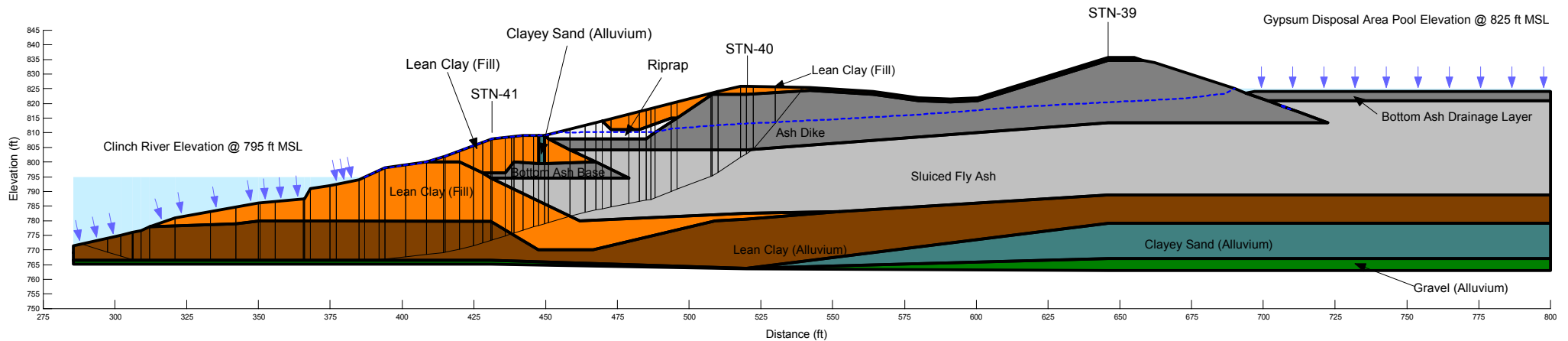


Note:
The results of the analysis shown here are based on available subsurface information, laboratory test results, and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Sluiced Fly Ash	105 pcf	100 psf	18.4 °
Lean Clay (Fill)	126 pcf	700 psf	17.6 °
Gravel (Alluvium)	135 pcf	100 psf	30 °
Clayey Sand (Alluvium)	112 pcf	100 psf	23 °
Lean Clay (Alluvium)	123 pcf	350 psf	21.1 °
Bottom Ash Base	105 pcf	0 psf	33 °
Ash Dike	105 pcf	0 psf	33 °
Rip Rap	115 pcf	0 psf	40 °

Factor of Safety: 1.0

Horizontal Seismic Coefficient $K_h = 0.131 \text{ g}$
2500 year Return Period Event



Date of Assessment - 11/4/2011

Project No. 175551015



Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants

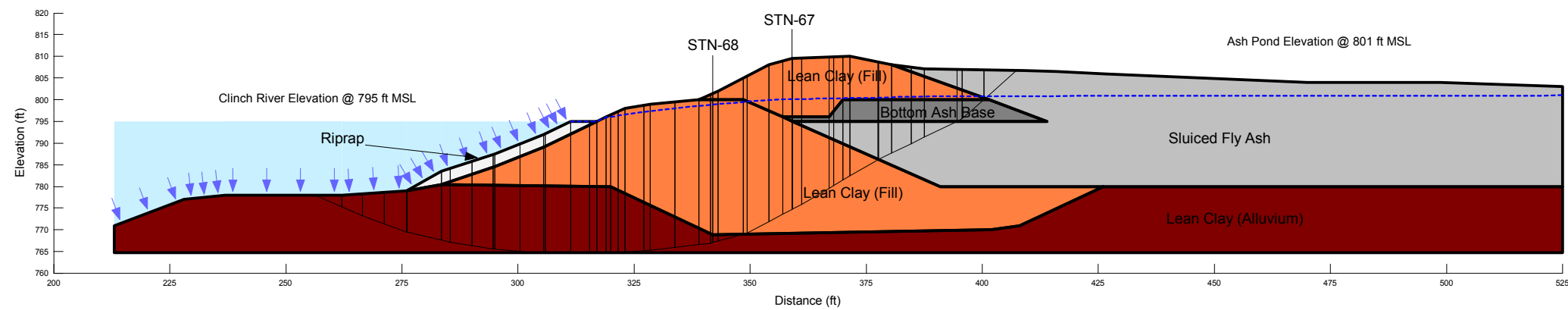
Section S - Fly Ash Disposal Area 2
Bull Run Fossil Plant
Clinton, Tennessee

Note:
 The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Sluiced Fly Ash	105 pcf	100 psf	18.4 °
Lean Clay (Fill)	126 pcf	700 psf	17.6 °
Lean Clay (Alluvium)	123 pcf	350 psf	21.1 °
Bottom Ash Base	105 pcf	0 psf	33 °
Rip-Rap	105 pcf	0 psf	40 °

Additional remediation measures taken from URS plans dated 08/13/2010

Factor of Safety: 1.4
 Horizontal Seismic Coefficient K_h = Value: 0.131 g
 2500-year Return Period Event



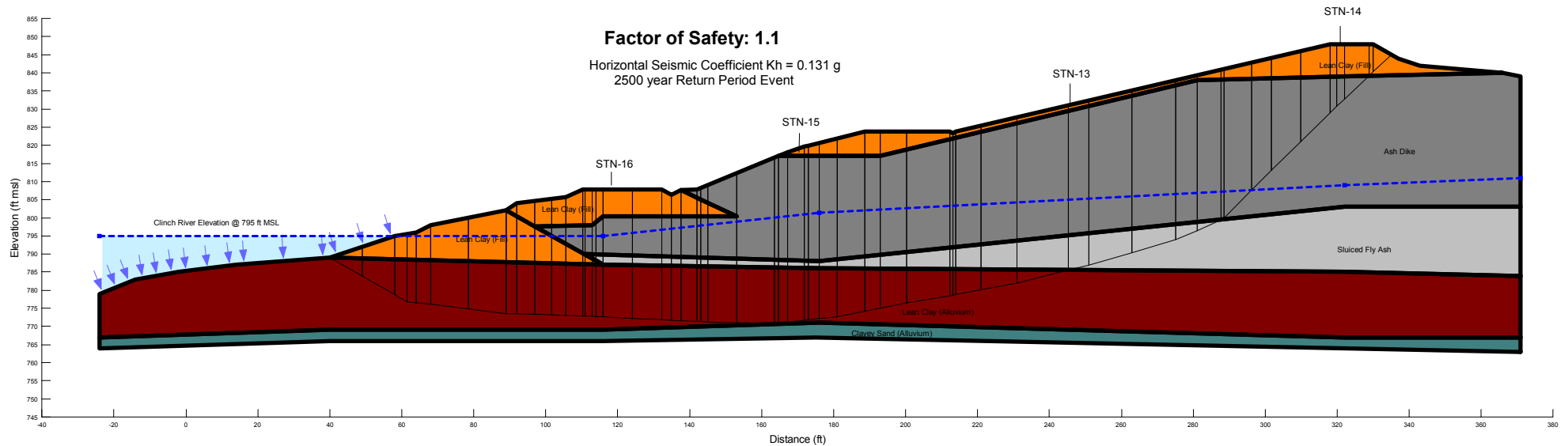
DRAFT
Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants

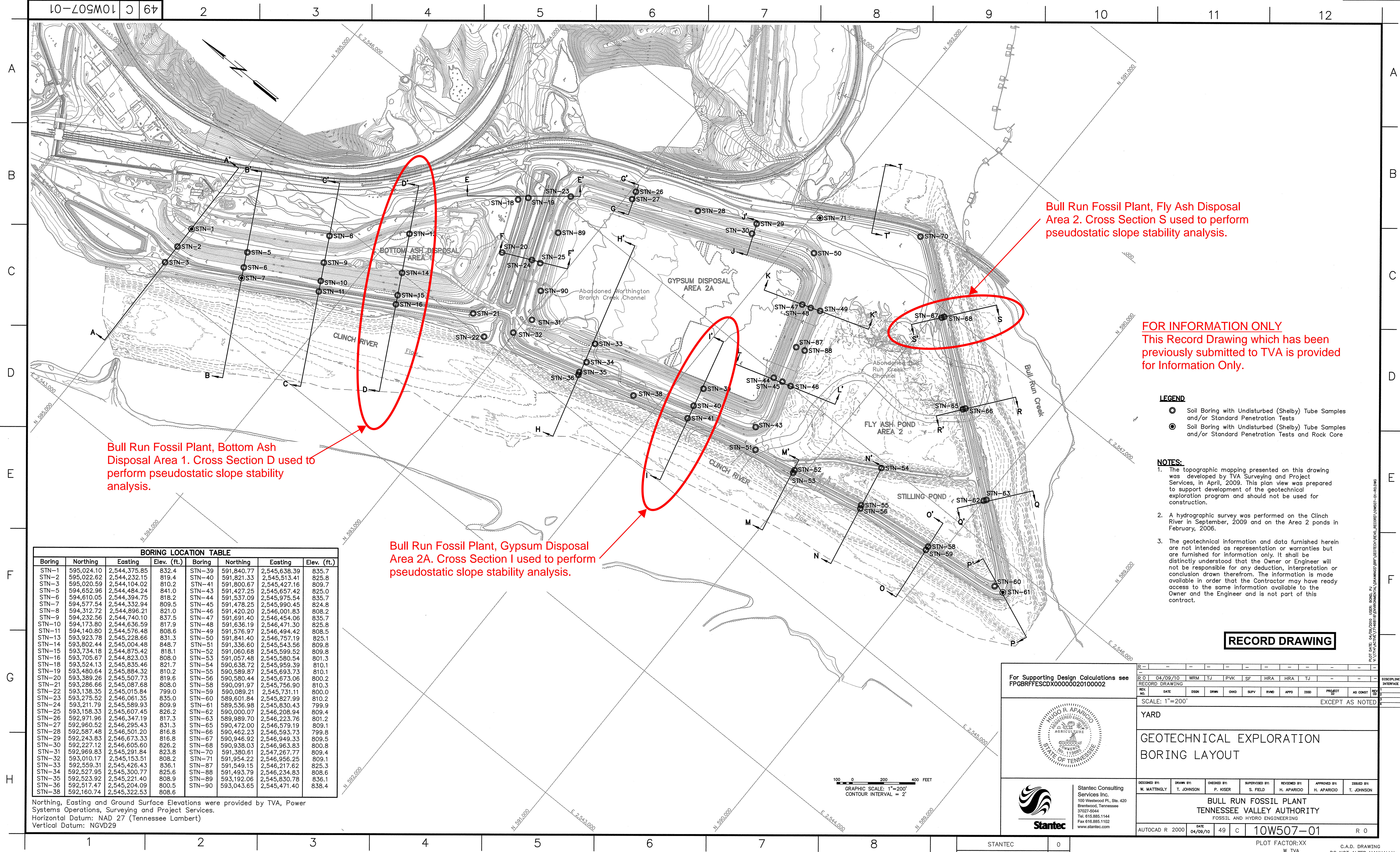
Section D - Bottom Ash Disposal Area 1
Bull Run Fossil Plant
Clinton, Tennessee



Note:
The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Lean Clay (Fill) - Lower Confinement	126	1000 psf	0 °
Lean Clay (Fill) - Higher Confinement	126	50 psf	17.6 °
Ash Dike	105	0 psf	33 °
Sluiced Ash	105	100 psf	18.4 °
Lean Clay (Alluvium) - Lower Confinement	123	528 psf	0 °
Lean Clay (Alluvium) - Higher Confinement	123	20.5 psf	21.1 °
Clayey Sand (Alluvium)	112	100 psf	23 °





Colbert Fossil Plant (COF)

Pseudostatic Slope Stability Analysis

CCP Storage Facilities - Existing Conditions

Tennessee Valley Authority Fossil Plants



Section I - Disposal Area 5

Colbert Fossil Plant

Tuscumbia, Alabama

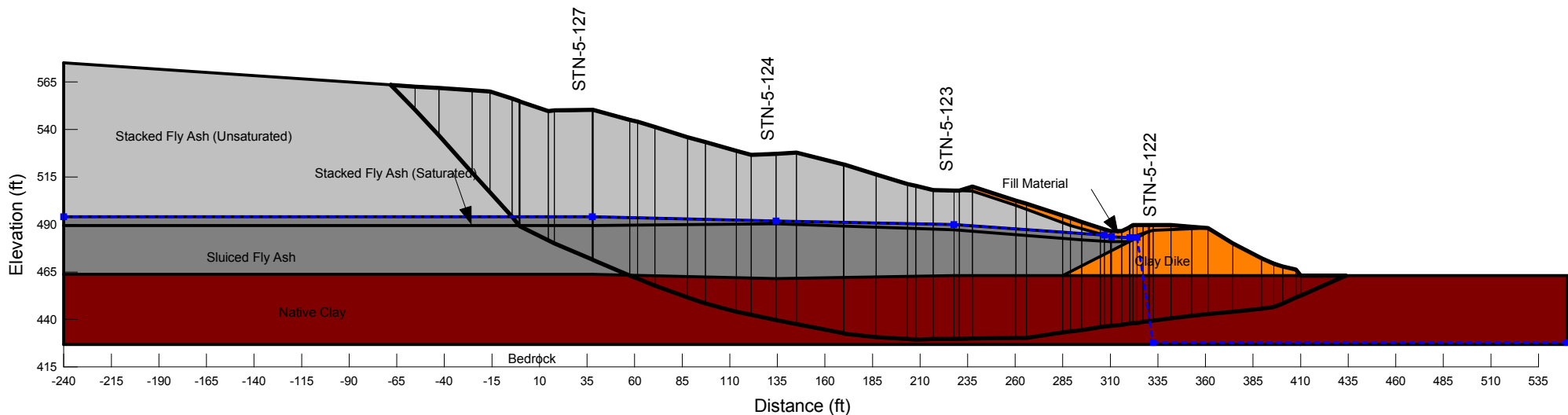
Material Type	Unit Weight	Cohesion	Friction Angle
Stacked Fly Ash (Unsaturated)	105 pcf	0 psf	32 °
Stacked Fly Ash (Saturated)	105 pcf	0 psf	32 °
Sluiced Fly Ash	85 pcf	400 psf	15 °
Fill Material	125 pcf	200 psf	19 °
Clay Dike	125 pcf	200 psf	19 °
Native Clay	125 pcf	290 psf	19 °

Note:
The results of analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Additional remediation measures taken from URS plans dated 7/09/2010.

Factor of Safety: 1.0

Horizontal Sesmic Coefficient $K_h = 0.138\text{ g}$
2500-year Return Period Event



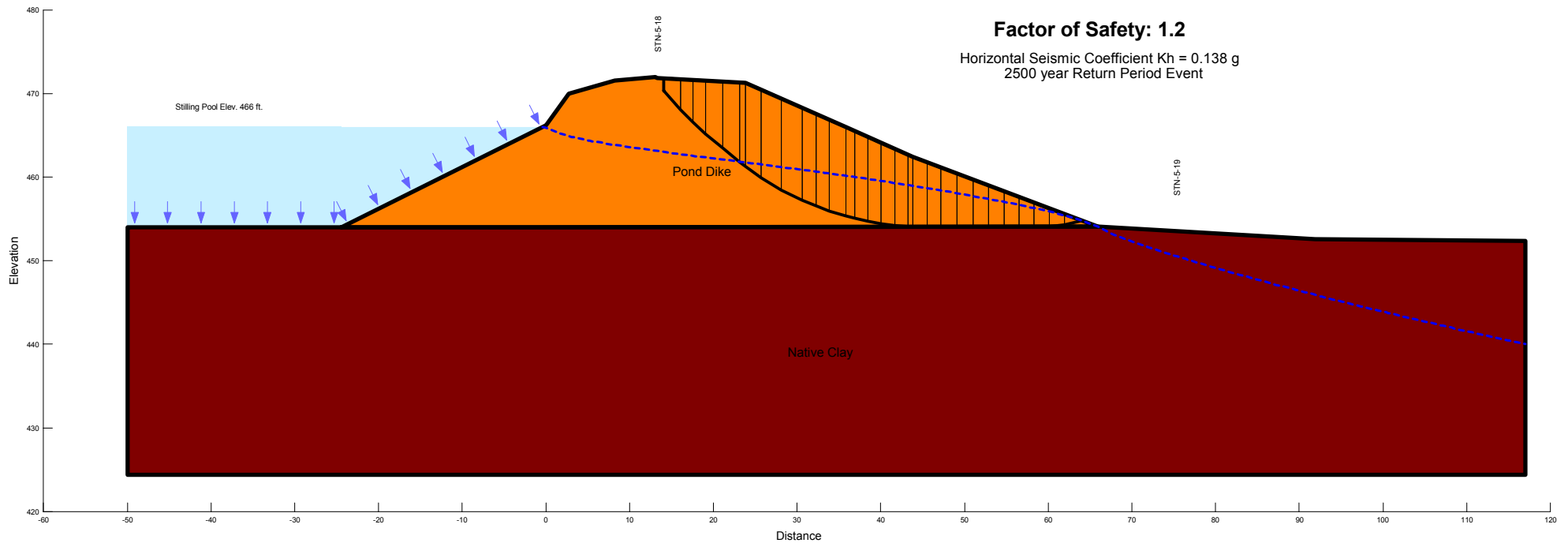
**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**

**Section J - Disposal Area 5 Stilling Basin
Colbert Fossil Plant
Tuscumbia, Alabama**



Note:
The results and analysis shown here are based on available subsurface information, laboratory test results, and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Native Clay	125 pcf	290 psf	19 °
Pond Dike	125 pcf	200 psf	19 °



**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**



**Section D - Ash Pond 4
Colbert Fossil Plant
Tennessee Valley Authority**

Note:

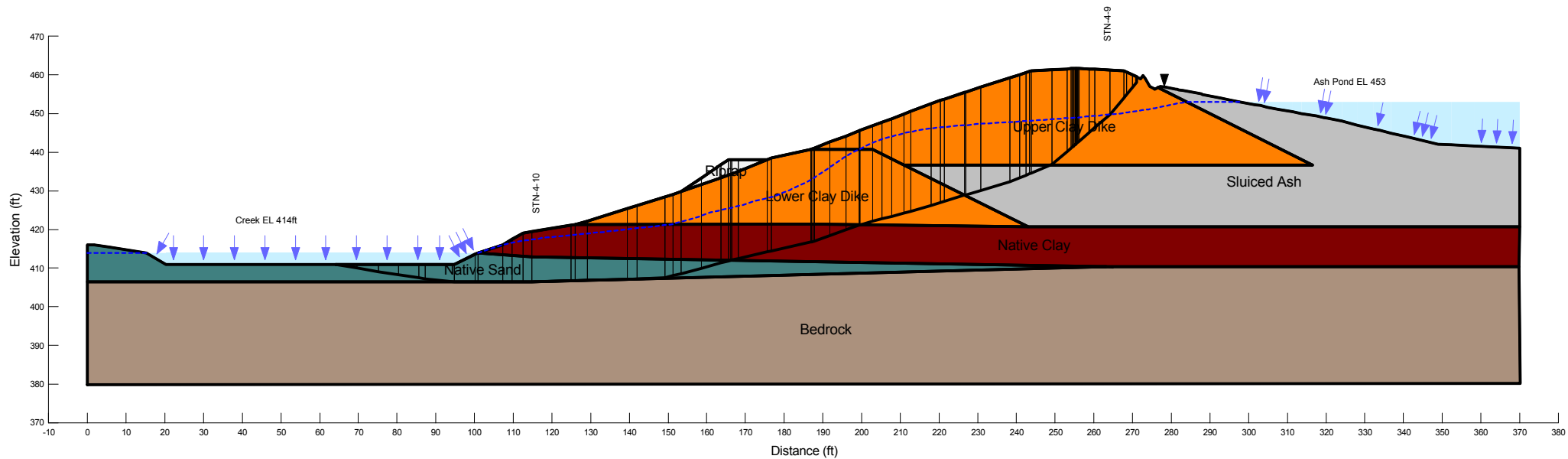
The results of the analysis shown here are based on available subsurface information, laboratory test results, and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

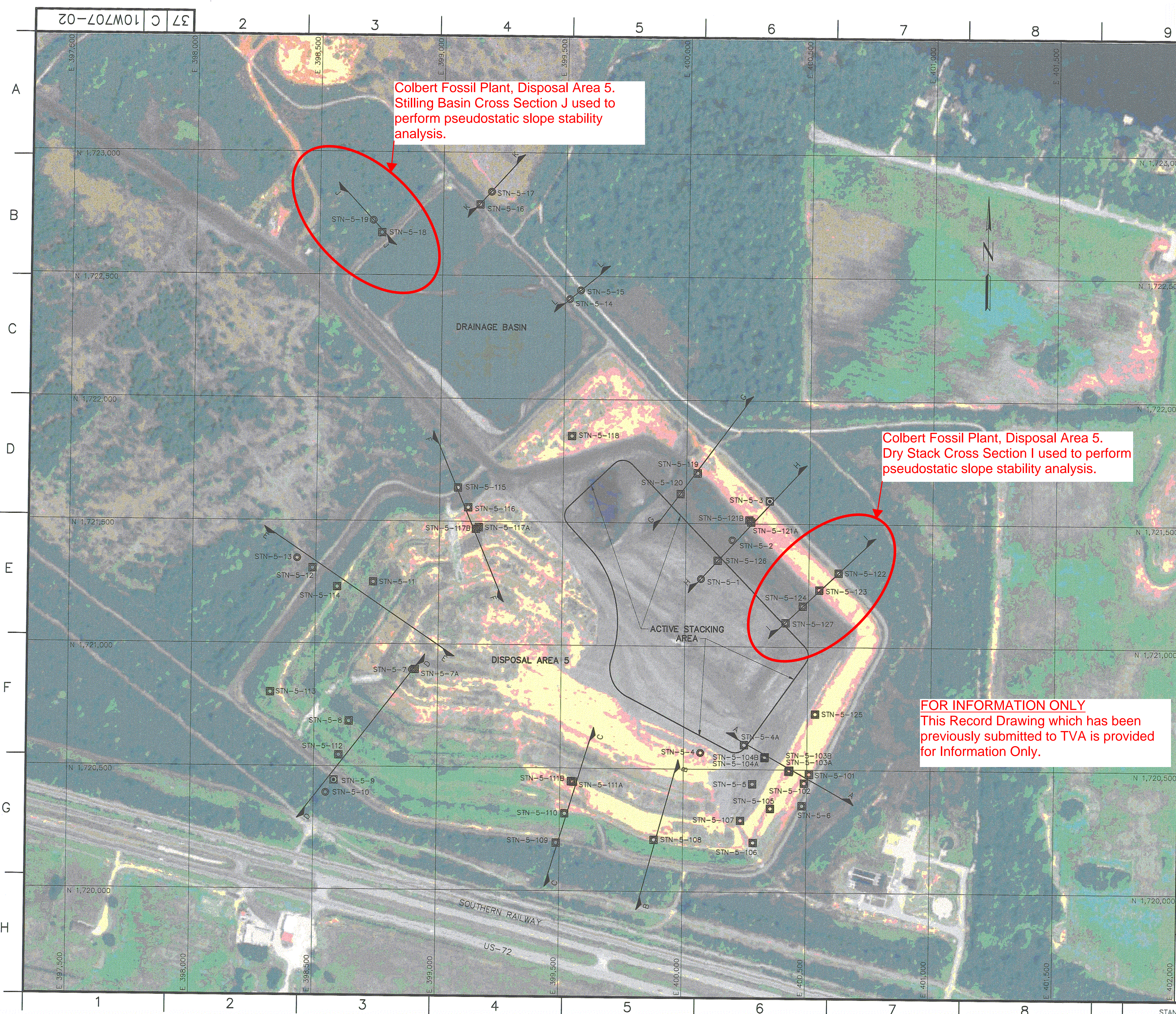
Additional remediation measures taken from URS plans dated 05/20/2010.

Material Type	Unit Weight	Cohesion	Friction Angle
Native Clay	129 pcf	700 psf	14 °
Upper Clay Dike	126 pcf	750 psf	12 °
Lower Clay Dike	127 pcf	400 psf	14 °
Sluiced Ash	85 pcf	400 psf	10 °
Native Sand	110 pcf	0 psf	30 °

Factor of Safety: 1.0

Horizontal Seismic Coefficient $K_h = 0.138 \text{ g}$
2500-year Return Period Event





Colbert Fossil Plant, Disposal Area 5.
Stilling Basin Cross Section J used to
perform pseudostatic slope stability
analysis.

Colbert Fossil Plant, Disposal Area 5.
Dry Stack Cross Section I used to perform
pseudostatic slope stability analysis.

FOR INFORMATION ONLY
This Record Drawing which has been
previously submitted to TVA is provided
for Information Only.

LEGEND

- Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Sampling
 - Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling (Piezometer Installed)
 - ⊗ Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Sampling, Rock Core, And Piezometer Installed
- 7 7
Cross Section

BORING LOCATION TABLE

Boring	Northing	Eastng	Elev. (ft.)
STN-5-1	1,721,274.40	400,063.00	552.91
STN-5-2	1,721,432.78	400,188.85	529.19
STN-5-3	1,721,592.79	400,338.67	487.54
STN-5-4	1,720,567.21	400,067.66	559.98
STN-5-4A	1,720,600.17	400,244.82	550.70
STN-5-5	1,720,441.17	400,279.90	529.71
STN-5-6	1,720,353.73	400,481.02	488.06
STN-5-7	1,720,895.57	398,896.12	564.52
STN-5-7A	1,720,897.20	398,906.71	565.34
STN-5-8	1,720,684.92	398,641.15	517.92
STN-5-9	1,720,444.22	398,582.58	480.52
STN-5-10	1,720,393.13	398,549.64	459.65
STN-5-11	1,721,251.58	398,733.51	515.53
STN-5-12	1,721,306.21	398,486.91	477.77
STN-5-13	1,721,346.27	398,424.36	452.26
STN-5-14	1,722,407.30	399,518.43	475.88
STN-5-15	1,722,443.72	399,562.30	462.85
STN-5-16	1,722,788.34	399,149.51	475.84
STN-5-17	1,722,840.75	399,195.10	458.60
STN-5-18	1,722,671.43	398,752.90	472.18
STN-5-19	1,722,721.58	398,717.11	453.41
STN-5-101	1,720,482.33	400,509.25	486.97
STN-5-102	1,720,445.72	400,489.29	486.80
STN-5-103A	1,720,493.35	400,426.84	508.95
STN-5-103B	1,720,498.42	400,429.29	509.48
STN-5-104A	1,720,548.28	400,328.22	530.46
STN-5-104B	1,720,551.74	400,330.04	530.75
STN-5-105	1,720,341.58	400,353.20	507.40
STN-5-106	1,720,202.10	400,284.88	505.40
STN-5-107	1,720,292.02	400,232.46	527.19
STN-5-108	1,720,211.06	399,882.93	509.29
STN-5-109	1,720,195.76	399,486.17	481.82
STN-5-110	1,720,315.43	399,519.59	509.44
STN-5-111A	1,720,446.15	399,553.23	529.32
STN-5-111B	1,720,446.66	399,545.11	529.34
STN-5-112	1,720,546.53	398,601.38	497.91
STN-5-113	1,720,800.24	398,321.28	499.24
STN-5-114	1,721,230.71	398,587.74	497.57
STN-5-115	1,721,636.94	399,073.55	477.77
STN-5-116	1,721,557.19	399,115.86	491.53
STN-5-117A	1,721,475.92	399,159.40	512.79
STN-5-117B	1,721,469.59	399,148.35	512.84
STN-5-118	1,721,850.73	399,533.21	499.18
STN-5-119	1,721,703.92	400,044.78	504.54
STN-5-120	1,721,619.44	399,976.15	525.76
STN-5-121A	1,721,506.28	400,264.15	508.23
STN-5-121B	1,721,514.55	400,256.18	507.94
STN-5-122	1,721,301.29	400,620.41	486.86
STN-5-123	1,721,231.75	400,542.54	508.11
STN-5-124	1,721,165.96	400,476.56	527.23
STN-5-125	1,720,727.47	400,531.28	509.03
STN-5-126	1,721,349.47	400,129.73	554.68
STN-5-127	1,721,097.71	400,407.87	550.45

RECORD DRAWING

75 0 150 300 FEET
GRAPHIC SCALE: 1"=150'

For Supporting Design Calculations see
FPGCOFFESC000000020100002



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DESIGNED BY:	DATE:	DRAWN BY:	CHECKED BY:	APPROVED BY:	ISSUED BY:
P. COOPER	03/24/10	R. PETTY	P. COOPER	R. ROBERTS	T. JOHNSON

YARD
DISPOSAL AREA NO. 5

GEOTECHNICAL EXPLORATION
BORING AND STABILITY
CROSS SECTION PLAN

COLBERT FOSSIL PLANT
TENNESSEE VALLEY AUTHORITY
FOSSIL AND HYDRO ENGINEERING

AUTOCAD R 2000 DATE 03/24/10 37 C 10W707-02 R 0

STANTEC 0
TASK COMPLETED BY: REV NO.

PLOT FACTOR:XX
W_TVA
C.A.D. DRAWING
DO NOT ALTER MANUALLY

A

B

C

D

E

F

G

H

BORING LOCATION TABLE			
BORING	NORTHING	EASTING	ELEVATION
STN-4-1	1,723,599.52	394,359.89	460.2
STN-4-2	1,723,632.72	394,289.32	439.4
STN-4-3	1,723,645.94	394,253.91	427.3
STN-4-4	1,723,316.42	394,738.76	460.4
STN-4-5	1,723,366.01	394,798.52	439.5
STN-4-6	1,723,373.16	394,864.54	419.8
STN-4-7	1,722,880.08	394,960.81	460.8
STN-4-8	1,722,943.49	395,089.90	421.6
STN-4-9	1,722,306.36	395,260.37	461.2
STN-4-10	1,722,357.09	395,401.10	420.9
STN-4-11	1,721,882.96	395,485.31	461.3
STN-4-12	1,721,504.87	395,746.27	446.2
STN-4-13	1,721,330.83	395,874.15	425.3
STN-4-14	1,721,420.08	395,715.53	461.9
STN-4-15	1,721,219.13	395,347.89	460.5
STN-4-16	1,721,126.23	395,351.96	435.1
STN-4-17	1,721,539.59	394,582.90	476.8
STN-4-18	1,721,402.10	394,555.64	461.1
STN-4-19	1,721,352.01	394,475.66	440.3
STN-4-20	1,721,987.93	394,461.02	482.3
STN-4-21	1,721,985.14	394,262.71	460.2
STN-4-22	1,721,957.70	394,065.63	438.0
STN-4-23	1,722,728.47	394,227.94	461.1
STN-4-24	1,722,823.19	394,138.84	444.5

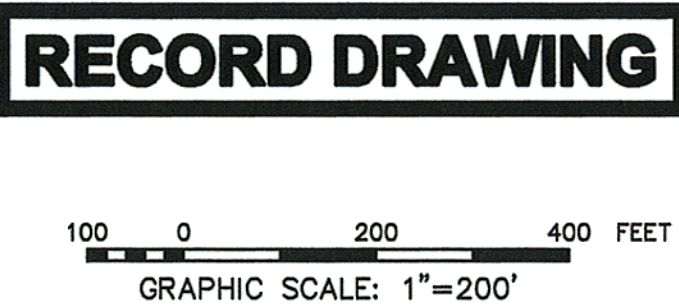


Colbert Fossil Plant, Ash Pond 4.
Cross Section D used to perform
pseudostatic slope stability analysis.

- LEGEND**
- Soil Boring With Continuous Standard Penetration Tests And/OR Shelby Tube Sampling
 - Soil Boring With Continuous Standard Penetration Tests And/OR Shelby Tube Piston Sampling, And Piezometer Installation
 - Soil Boring With Continuous Standard Penetration Tests And/or Shelby Tube Sampling, Rock Core, And Piezometer Installation

- NOTES**
- Topographic and survey information provided by the Tennessee Valley Authority.
 - The Boring Logs And Related Information Shown On This Drawing Depict Approximate Subsurface Conditions Only At The Specific Boring Locations Noted And At The Time Of Drilling. Conditions At Other Locations May Differ From Those Occurring At The Boring Locations. Also, The Passage Of Time May Result In A Change In The Subsurface Conditions At The Boring Locations. Any Correlations Shown Between Borings Are Generally Based On Straight Line Interpolation. Actual Conditions Between Borings Are Unknown And May Differ From Those Shown.

FOR INFORMATION ONLY
This Record Drawing which has been
previously submitted to TVA is provided
for Information Only.



For Supporting Design Calculations see FPGCOFFESC00000002010001		<table><tr><td>R</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td></tr><tr><td>R</td><td>O</td><td>01/22/10</td><td>PC</td><td>SB</td><td>PC</td><td>RLR</td><td>RLR</td><td>RLR</td><td>TJ</td><td>-</td><td>-</td></tr><tr><td>REV</td><td>NO.</td><td>DATE</td><td>DSGN</td><td>DRWN</td><td>CHKD</td><td>SUPV</td><td>RWDG</td><td>APPR</td><td>ISSD</td><td>PROJECT</td><td>AS CONST</td></tr><tr><td colspan="11">SCALE: 1"=200'</td></tr><tr><td colspan="11">EXCEPT AS NOTED</td></tr></table>										R	-	-	-	-	-	-	-	-	-	-	-	R	O	01/22/10	PC	SB	PC	RLR	RLR	RLR	TJ	-	-	REV	NO.	DATE	DSGN	DRWN	CHKD	SUPV	RWDG	APPR	ISSD	PROJECT	AS CONST	SCALE: 1"=200'											EXCEPT AS NOTED										
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		YARD ASH POND 4 GEOTECHNICAL EXPLORATION BORING AND STABILITY CROSS-SECTION PLAN																																																																			
		<table><tr><td>DESIGNED BY:</td><td>DRAWN BY:</td><td>CHECKED BY:</td><td>SUPERVISED BY:</td><td>REVIEWED BY:</td><td>APPROVED BY:</td><td>ISSUED BY:</td></tr><tr><td>P. COOPER</td><td>S. BRADSHAW</td><td>P. COOPER</td><td>R. ROBERTS</td><td>R. ROBERTS</td><td>R. ROBERTS</td><td>T. JOHNSON</td></tr></table>										DESIGNED BY:	DRAWN BY:	CHECKED BY:	SUPERVISED BY:	REVIEWED BY:	APPROVED BY:	ISSUED BY:	P. COOPER	S. BRADSHAW	P. COOPER	R. ROBERTS	R. ROBERTS	R. ROBERTS	T. JOHNSON																																												
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P. COOPER	S. BRADSHAW	P. COOPER	R. ROBERTS	R. ROBERTS	R. ROBERTS	T. JOHNSON																																																															
Stantec Consulting Services Inc. 1801 Nelson Miller Pkwy. Louisville, Kentucky 40223-2177 Tel. 502.212.5000 Fax 502.212.5005 www.stantec.com		COLBERT FOSSIL PLANT TENNESSEE VALLEY AUTHORITY FOSSIL AND HYDRO ENGINEERING																																																																			
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STANTEC	0
TASK COMPLETED BY:	REV NO.

Gallatin Fossil Plant (GAF)

**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**

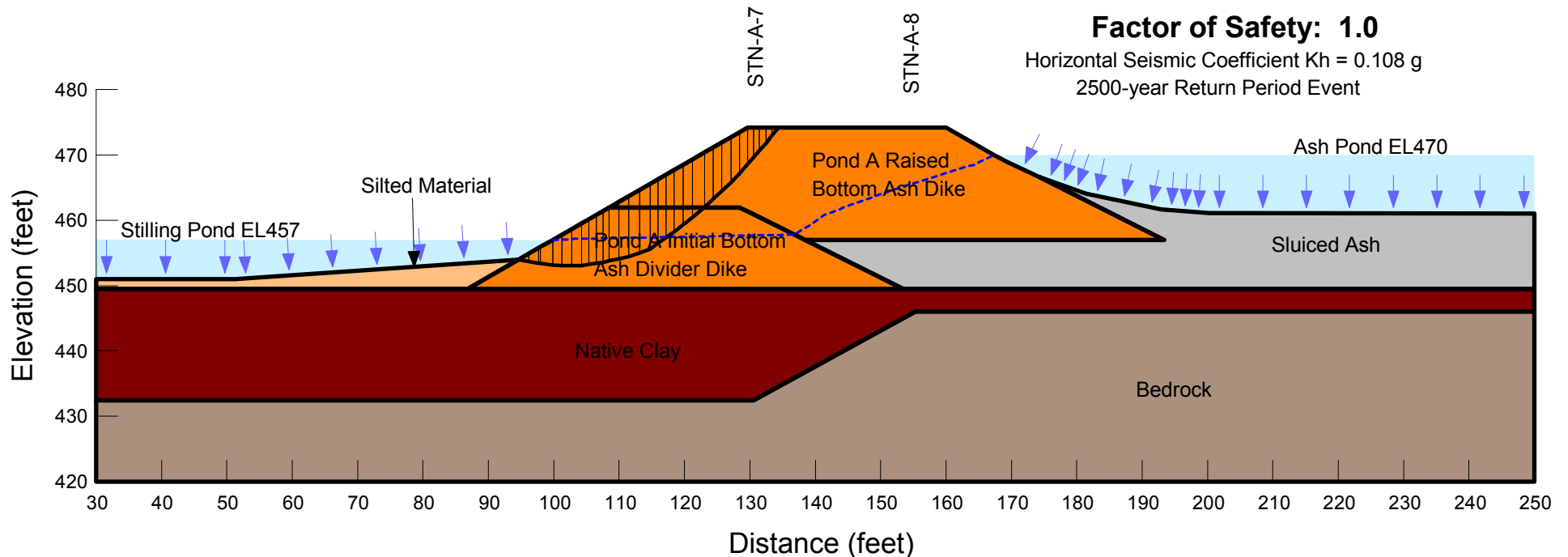


**Section K - Ash Pond A
Gallatin Fossil Plant
Gallatin, Tennessee**

Material Type	Unit Weight	Cohesion	Friction Angle
Pond A Initial Bottom Ash Divider Dike	105 pcf	0 psf	33 °
Pond A Raised Bottom Ash Dike	105 pcf	0 psf	34 °
Sluiced Ash	85 pcf	400 psf	10 °
Native Clay	125 pcf	550 psf	13 °
Silted Material	85 pcf	400 psf	10 °

Note:

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Pseudostatic Slope Stability Analysis CCP Storage Facilities - Existing Conditions Tennessee Valley Authority Fossil Plants



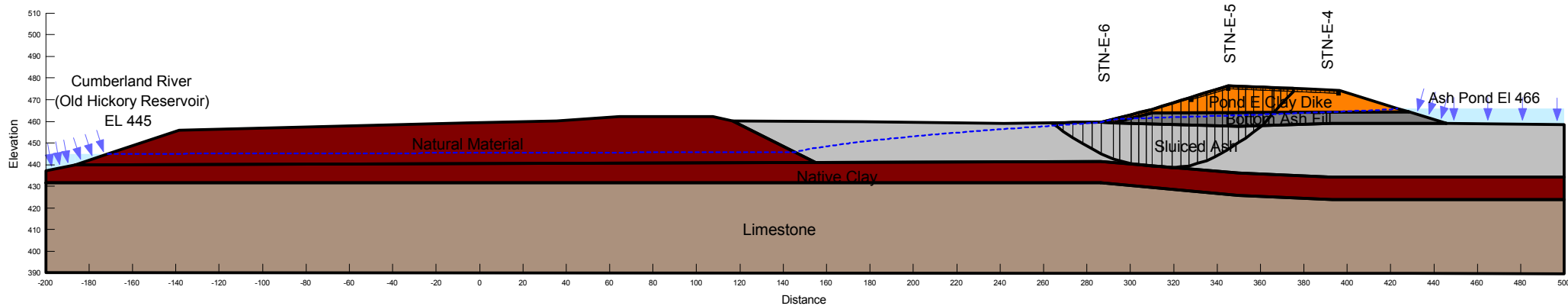
Section B - Ash Pond E Gallatin Fossil Plant Gallatin, Tennessee

Note:

The results of analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Pond E Clay Dike	125 pcf	400 psf	15 °
Bottom Ash Fill	100 pcf	0 psf	34 °
Sluiced Ash	85 pcf	400 psf	10 °
Native Clay	125 pcf	550 psf	13 °

Factor of Safety: 1.3
Horizontal Seismic Coefficient $K_h = 0.108 g$
2500-year Return Period Event



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Gallatin Fossil Plant, Ash Pond A. Cross Section K used to perform pseudostatic slope stability analysis.

LEGEND

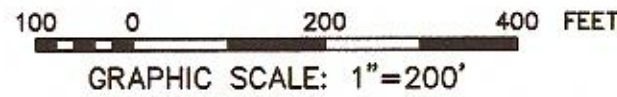
- Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling
 - Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling And Rock Core
 - Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling And Piezometer Location
 - Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling And Rock Core And Piezometer Location
- A A' Cross Section

NOTES

- Topographic and survey information provided by the Tennessee Valley Authority.
- The boring logs and related information shown on this drawing depict approximate subsurface conditions only at the specific boring locations noted and at the time of drilling. Conditions at other locations may differ from those occurring at the boring locations. Also, the passage of time may result in a change in the subsurface conditions at the boring locations. Any correlations shown between borings are generally based on straight line interpolation. Actual conditions between borings are unknown and may differ from those shown.

BORING LOCATION TABLE			
BORING	NORTHING	EASTING	ELEVATION (ft.)
STN-A-1	707,019.68	1,879,799.57	472.8
STN-A-2	706,994.16	1,879,810.94	473.3
STN-A-3	707,510.75	1,880,731.90	472.9
STN-A-4	707,498.65	1,880,758.47	473.8
STN-A-5	708,368.74	1,881,417.01	473.7
STN-A-6	708,353.42	1,881,433.71	474.0
STN-A-7	708,921.58	1,881,894.55	474.5
STN-A-8	708,907.06	1,881,914.61	474.8
STN-A-9	709,132.64	1,882,470.74	472.4
STN-A-10	709,085.67	1,882,461.16	474.1
STN-C-1	707,402.48	1,879,680.01	462.0

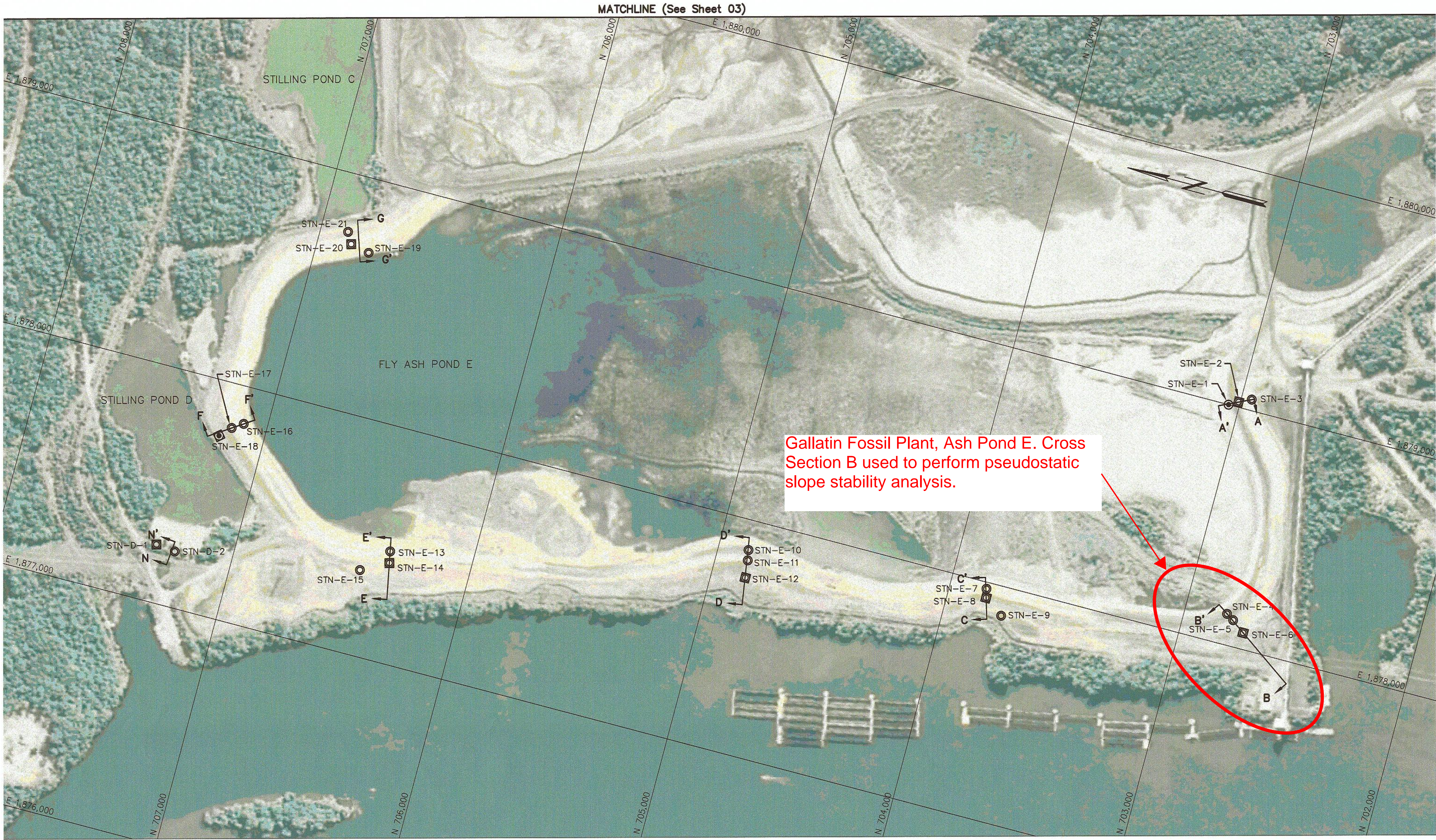
RECORD DRAWING



FOR INFORMATION ONLY
This Record Drawing which has been previously submitted to TVA is provided for Information Only.

For Supporting Design Calculations see FPGGAFFESCDX00000020100001		R 0 05/27/10 PC RP PC RLR RLR RLR TJ										DISCIPLINE INTERFACE	
RECORD DRAWING		REV. NO. DATE DESIG. DRWN. CHG. SUPV. RVD. APPD. ISSD. PROJECT NO. AS CONST. REV. BY										EXCEPT AS NOTED	
		YARD ASH POND/STILLING POND COMPLEX											
		GEOTECHNICAL EXPLORATION BORING LAYOUT											
DESIGNED BY: P. COOPER		DRAWN BY: R. PETTY		CHECKED BY: P. COOPER		SUPERVISED BY: R. ROBERTS		REVIEWED BY: R. ROBERTS		APPROVED BY: R. ROBERTS		ISSUED BY: T. JOHNSON	
		GALLATIN FOSSIL PLANT TENNESSEE VALLEY AUTHORITY FOSSIL AND HYDRO ENGINEERING											
		AUTOCAD R 2000 DATE 05/27/10 39 C 10W504-03 R 0											

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Gallatin Fossil Plant, Ash Pond E. Cross Section B used to perform pseudostatic slope stability analysis.

LEGEND

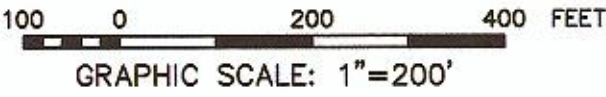
- Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling
- Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling And Rock Core
- Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling And Piezometer Location
- Soil Boring With Continuous Standard Penetration Tests And/Or Shelby Tube Piston Sampling And Rock Core And Piezometer Location

NOTES

- Topographic and survey information provided by the Tennessee Valley Authority.
- The boring logs and related information shown on this drawing depict approximate subsurface conditions only at the specific boring locations noted and at the time of drilling. Conditions at other locations may differ from those occurring at the boring locations. Also, the passage of time may result in a change in the subsurface conditions at the boring locations. Any correlations shown between borings are generally based on straight line interpolation. Actual conditions between borings are unknown and may differ from those shown.

BORING LOCATION TABLE			
BORING	NORTHING	EASTING	ELEVATION (ft.)
STN-D-1	707,328.99	1,877,246.92	460.8
STN-D-2	707,245.18	1,877,237.96	460.4
STN-E-1	703,045.88	1,879,000.10	474.1
STN-E-2	703,007.37	1,879,022.21	475.7
STN-E-3	702,955.21	1,879,046.66	459.6
STN-E-4	702,820.82	1,878,131.27	474.3
STN-E-5	702,788.65	1,878,111.48	476.1
STN-E-6	702,733.38	1,878,070.14	459.6
STN-E-7	703,843.80	1,877,971.87	475.1
STN-E-8	703,835.47	1,877,934.64	476.5
STN-E-9	703,753.39	1,877,876.25	451.8
STN-E-10	704,870.32	1,877,862.37	474.9
STN-E-11	704,863.36	1,877,828.40	476.1
STN-E-12	704,854.47	1,877,754.46	455.3
STN-E-13	706,353.41	1,877,474.21	474.3
STN-E-14	706,343.79	1,877,425.50	477.0
STN-E-15	706,458.09	1,877,364.00	463.4
STN-E-16	707,101.38	1,877,842.04	474.9
STN-E-17	707,146.54	1,877,811.85	475.4
STN-E-18	707,190.77	1,877,765.92	461.6
STN-E-19	706,774.43	1,878,687.08	472.8
STN-E-20	706,856.53	1,878,704.54	476.0
STN-E-21	706,883.00	1,878,751.72	461.6

RECORD DRAWING



FOR INFORMATION ONLY
This Record Drawing which has been previously submitted to TVA is provided for Information Only.

For Supporting Design Calculations see FPGGAFFESCDX0000020100001		R 0 05/27/10 PC RP PC RLR RLR RLR TJ - - -											
RECORD DRAWING		REV. NO. DATE DESN DRWN CHD SUPV RYMD APPD USD PROJECT ID AS CONST REV											
SCALE: 1"=200'		EXCEPT AS NOTED											
YARD ASH POND/STILLING POND COMPLEX													
GEOTECHNICAL EXPLORATION BORING LAYOUT													
DESIGNED BY: P. COOPER		DRAWN BY: R. PETTY		CHECKED BY: P. COOPER		SUPERVISED BY: R. ROBERTS		REVIEWED BY: R. ROBERTS		APPROVED BY: R. ROBERTS		ISSUED BY: T. JOHNSON	
STANTEC Consulting Services Inc. 1901 Nelson Miller Pky. Louisville, Kentucky 40223-2177 Tel. 502.212.5000 Fax 502.212.5055 www.stantec.com		GALLATIN FOSSIL PLANT TENNESSEE VALLEY AUTHORITY FOSSIL AND HYDRO ENGINEERING											
AUTOCAD R 2000		DATE 05/27/10		39 C		10W504-02		R 0					

John Sevier Fossil Plant (JSF)

Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants

Section I - Bottom Ash Pond
John Sevier Plant
Rogersville, Tennessee

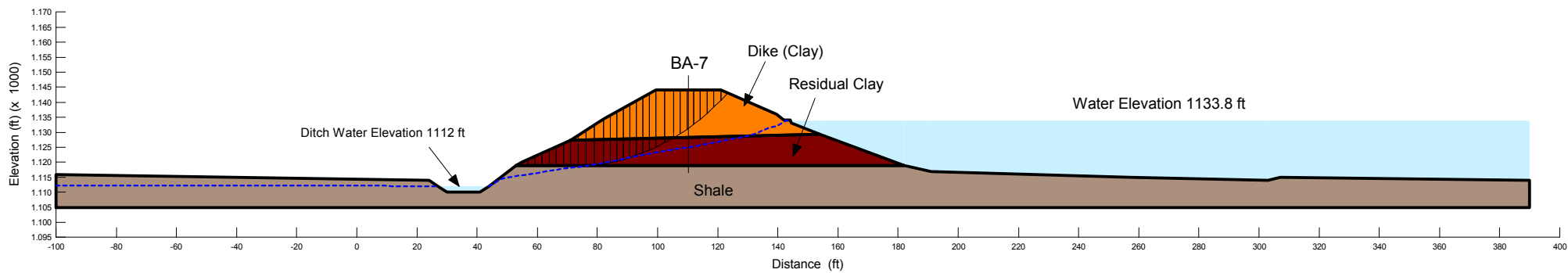


Note:
The results of analysis shown here are based on available subsurface information, laboratory test results, and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Dike (Clay)	126 pcf	715 psf	10.6 °
Residual Clay	120 pcf	1000 psf	11.6 °
Bedrock (Shale)	N/A	N/A	N/A

Factor of Safety: 2.2

Horizontal Seismic Coefficient $K_h = 0.115\text{ g}$
2500-year Return Period Event



10-705M01 C 17

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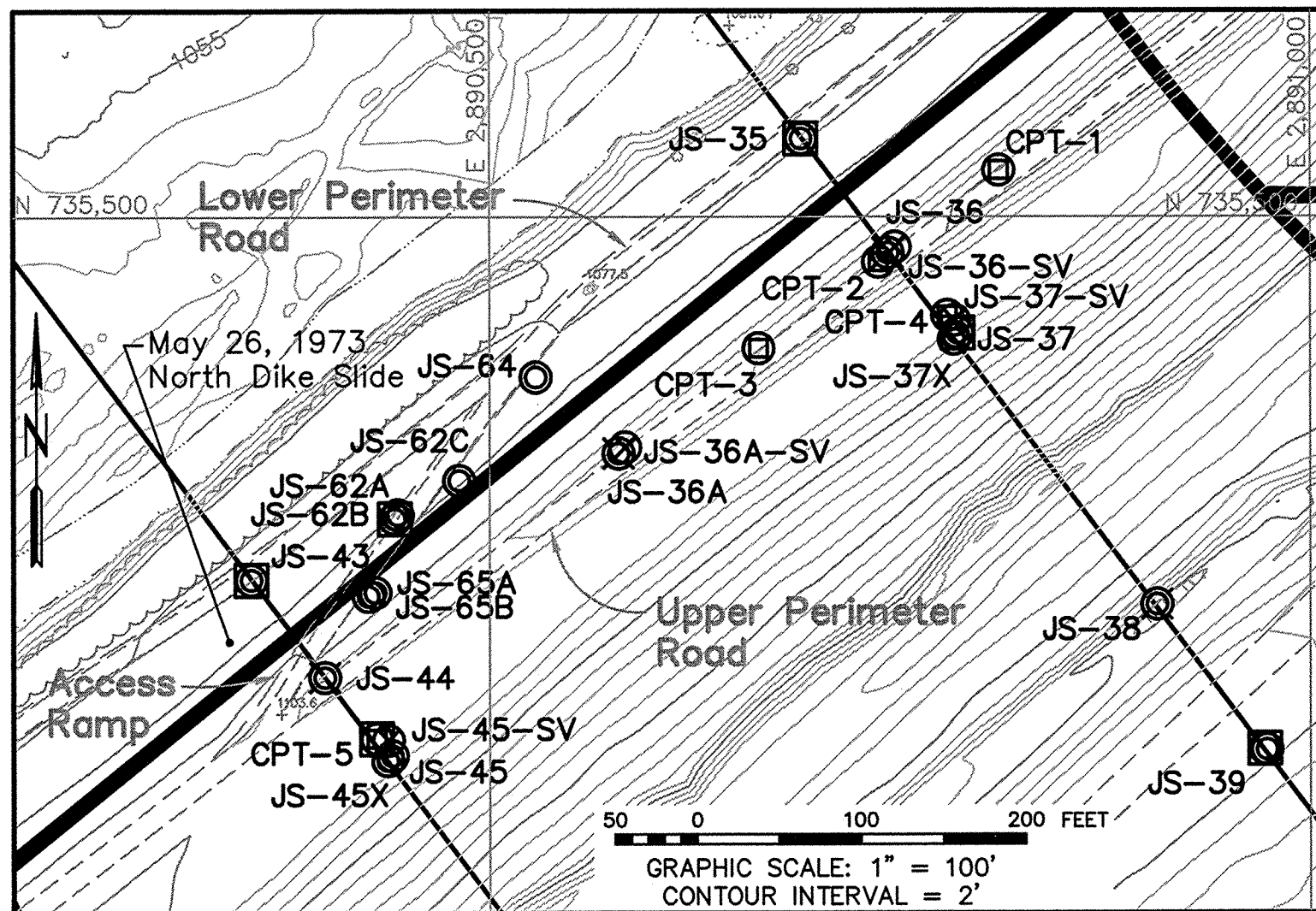
BORING LOCATION TABLE				BORING TYPE
BORING	NORTHING	EASTING	ELEV. (FT.)	
BA-1	734,343.87	2,893,639.94	1,145.4	Sample / Piezometer
BA-2	734,229.93	2,893,695.53	1,145.9	Sample / Piezometer
BA-3	733,939.03	2,893,286.73	1,145.3	Sample / Piezometer
BA-4	733,486.11	2,890,407.91	1,145.2	Sample
BA-5	733,604.48	2,889,750.33	1,144.9	Sample / Piezometer
BA-6	733,808.75	2,889,830.63	1,145.1	Sample
BA-7	733,872.97	2,890,492.40	1,144.3	Sample
BA-8	733,946.71	2,891,566.83	1,145.2	Sample / Piezometer
BA-9	734,027.41	2,892,632.01	1,144.7	Sample
JP-1	733,930.64	2,888,187.78	1,105.4	Sample
JP-2	733,703.71	2,887,641.90	1,105.7	Sample
JP-3	733,483.09	2,886,974.16	1,105.8	Sample / Piezometer
JP-4	733,323.27	2,886,353.14	1,105.6	Sample / Piezometer
JP-4A	733,325.38	2,886,401.23	1,105.3	Shelby Tubes
JP-5	732,679.06	2,886,045.57	1,104.5	Sample / Piezometer
JP-6	732,862.78	2,886,526.80	1,106.3	Sample / Piezometer
JS-10	736,877.33	2,892,782.32	1,085.0	Sample / Piezometer
JS-11	736,817.60	2,892,703.95	1,115.3	Sample / Slope Inclinator
JS-12	736,796.96	2,892,666.90	1,114.8	Sample / Piezometer
JS-13	736,741.69	2,892,570.62	1,132.5	Sample / Piezometer
JS-15	737,186.07	2,892,539.85	1,084.1	Sample / Piezometer
JS-16	737,079.51	2,892,528.69	1,115.7	Sample / Slope Inclinator
JS-17	737,004.19	2,892,496.33	1,114.5	Sample / Piezometer
JS-18	736,848.84	2,892,429.18	1,136.3	Sample / Piezometer
JS-19	736,913.99	2,891,993.30	1,077.3	Sample / Piezometer
JS-20	736,826.84	2,892,070.81	1,113.8	Sample / Slope Inclinator
JS-21	736,784.15	2,892,107.96	1,111.0	Sample / Piezometer
JS-22	736,662.66	2,892,209.60	1,134.7	Sample / Piezometer
JS-23	736,562.81	2,891,652.34	1,075.1	Sample / Piezometer
JS-24	736,463.59	2,891,743.40	1,113.4	Sample / Slope Inclinator
JS-25	736,417.96	2,891,781.01	1,108.1	Sample / Piezometer
JS-26	736,300.23	2,891,894.54	1,141.8	Sample / Slope Inclinator
JS-27	736,239.87	2,891,944.24	1,158.3	Sample / Temporary Piezometer
JS-28	736,010.84	2,891,175.23	1,074.5	Sample / Piezometer
JS-29	735,935.78	2,891,247.73	1,111.5	Sample / Slope Inclinator
JS-30	735,899.72	2,891,288.23	1,105.6	Sample / Piezometer
JS-31	735,755.45	2,891,418.56	1,151.1	Sample / Slope Inclinator
JS-32	735,765.70	2,891,431.00	1,150.6	Sample / Piezometer
JS-33A	735,606.67	2,891,439.21	1,152.4	Sample
JS-33B	735,313.55	2,891,533.03	1,155.3	Sample
JS-34A	735,400.64	2,891,943.07	1,156.4	Sample
JS-34B	735,181.98	2,891,694.15	1,156.3	Sample
JS-34C	735,045.58	2,892,079.28	1,120.4	Sample / Piezometer
JS-35	735,547.59	2,890,689.83	1,078.9	Sample / Piezometer
JS-36	735,478.03	2,890,742.60	1,108.5	Sample / Slope Inclinator
JS-36-SV	735,481.63	2,890,746.85	1,108.4	Shear Vane Test / Shelby Tubes
JS-36A	735,355.98	2,890,578.53	1,106.2	Sample / Slope Inclinator
JS-36A-SV	735,355.98	2,890,578.53	1,106.2	Shear Vane Test / Shelby Tubes
JS-36B	735,703.43	2,891,025.07	1,110.8	Sample / Slope Inclinator
JS-37	735,429.18	2,890,784.99	1,103.8	Piezometer
JS-37-SV	735,436.98	2,890,782.91	1,102.3	Shear Vane Test / Shelby Tubes
JS-37X	735,429.18	2,890,784.99	1,103.8	Sample
JS-38	735,263.83	2,890,906.40	1,151.5	Sample / Slope Inclinator
JS-39	735,175.12	2,890,973.42	1,181.3	Sample / Temporary Piezometer
JS-40	735,048.86	2,891,066.57	1,170.2	Sample
JS-41	734,877.51	2,891,195.60	1,154.6	Sample
JS-42	734,710.66	2,891,295.11	1,138.2	Sample / Piezometer
JS-43	735,279.02	2,890,354.76	1,081.5	Sample / Piezometer
JS-44	735,219.55	2,890,399.56	1,103.2	Sample / Slope Inclinator
JS-45	735,171.68	2,890,440.72	1,101.3	Piezometer
JS-45-SV	735,181.14	2,890,436.31	1,100.1	Shear Vane Test / Shelby Tubes
JS-45B	735,168.74	2,890,438.03	1,101.5	Sample
JS-46	735,006.11	2,890,560.28	1,144.7	Sample / Slope Inclinator
JS-47	735,013.36	2,890,001.65	1,078.2	Sample / Piezometer
JS-48	734,956.57	2,890,044.99	1,101.3	Sample / Slope Inclinator
JS-49	734,898.66	2,890,091.75	1,088.9	Sample / Piezometer
JS-50	734,760.24	2,890,196.57	1,138.7	Sample / Piezometer
JS-52	734,518.95	2,890,384.61	1,136.8	Sample / Piezometer
JS-53	734,742.01	2,889,577.25	1,081.4	Sample / Piezometer
JS-54	734,685.87	2,889,594.68	1,100.2	Sample / Slope Inclinator
JS-55	734,611.13	2,889,621.92	1,097.4	Sample / Piezometer
JS-56	734,506.50	2,889,656.35	1,131.0	Sample / Piezometer
JS-57	734,277.92	2,889,720.99	1,130.1	Sample / Piezometer
JS-58	734,222.32	2,889,559.16	1,100.2	Piezometer
JS-58X	734,224.38	2,889,557.53	1,100.1	Sample
JS-59	734,047.10	2,889,202.69	1,089.3	Sample / Piezometer
JS-60A	736,513.29	2,891,697.31	1,089.5	Sample
JS-60B	736,515.46	2,891,699.27	1,089.5	Shelby Tubes / Piezometer
JS-61A	735,980.74	2,891,206.58	1,089.7	Sample / Piezometer
JS-61B	735,978.47	2,891,204.07	1,089.1	Shelby Tubes
JS-62A	735,318.64	2,890,444.05	1,090.0	Sample
JS-62B	735,316.23	2,890,442.25	1,090.0	Shelby Tubes / Piezometer
JS-62C	735,339.49	2,890,481.47	1,088.2	Sample
JS-63A	734,983.93	2,890,020.63	1,089.4	Sample
JS-63B	734,987.69	2,890,023.29	1,089.4	Shelby Tubes / Piezometer
JS-64	735,402.40	2,890,528.11	1,082.3	Sample
JS-65A	735,271.28	2,890,430.29	1,095.1	Sample / Shelby Tubes
JS-65B	735,269.06	2,890,426.10	1,094.7	Shelby Tubes
CPT-1	735,528.42	2,890,509.86	1,109.5	Cone Penetration Test
CPT-2	735,472.49	2,890,736.90	1,108.3	Cone Penetration Test
CPT-3	735,419.93	2,890,663.93	1,107.1	Cone Penetration Test
CPT-4	735,439.57	2,890,778.44	1,101.8	Cone Penetration Test
CPT-5	735,182.18	2,890,431.15	1,100.0	Cone Penetration Test

LEGEND

- Edge of River Pool
- Assumed Limits of Original Disposal Ponds
- Original Disposal Pond Designation
- Soil Boring With Standard Penetration Tests
- Soil Boring With Standard Penetration Tests (Piezometer Installed)
- Soil Boring With Standard Penetration Tests (Slope Inclinator Installed)
- Cone Penetration Test
- Shear Vane Test

NOTES:

- Topographic mapping was developed by Tuck Mapping Solutions, Inc. on March 19, 2009.
- The Tennessee Valley Authority Surveying and Project Services performed a hydrographic survey on the Holston River on September 17, 2009 and on the Bottom Ash Disposal Area 2 on January 12, 2006.
- The location of shown areas of past disturbance are approximations based on previous inspection reports.
- The limits of the Original Disposal Ponds were approximated using drawing 10N295 and previous inspection reports.
- The geotechnical information and data furnished herein are not intended as representation or warranties but are furnished for information only. It shall be distinctly understood that the Owner or Engineer will not be responsible for any deduction, interpretation or conclusion drawn therefrom. The information is made available in order that the Contractor may have ready access to the same information available to the Owner and the Engineer and is not part of this contract.



INSERT

SCALE: 1"=100'

February 15, 1994
Dike-Sloughing at Toe
of Dike about 50 feet
West of Stilling Pond

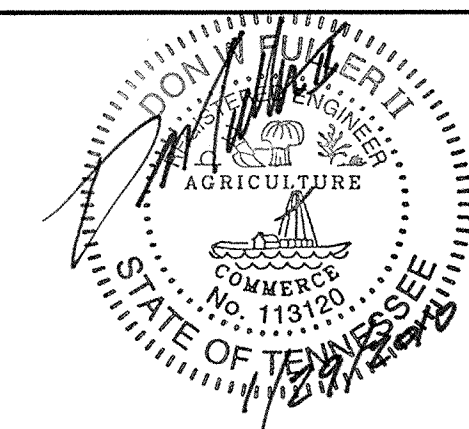
ASH DISPOSAL AREAS - BORING/INSTRUMENT PLAN

SCALE: 1"=200'

GRAPHIC SCALE: 1" = 200'
CONTOUR INTERVAL = 2'

RECORD DRAWING

For Supporting Design Calculations see
FPG JSF FES CDX 000 000 2010 0001



Stantec Consulting
Services Inc.
1409 N. Forbes Rd.
Lexington, Kentucky
40511-2059
Tel: 859.422.3000
Fax: 859.422.3100
www.stantec.com

REV.	DATE	DOGN	DRWN	CHKD	SUPV	RVMD	APPR	ISSD	PROJECT	AS CONST	REV
1	01/29/10	ARD	DMG	ARD	DLB	DLB	TJ				

SCALE: 1" = 200' EXCEPT AS NOTED

YARD
GEOTECHNICAL EXPLORATION
ASH DISPOSAL AREAS
BORING/INSTRUMENT PLAN

DESIGNED BY: A. DAVIS
DRAWN BY: D. GRAHAM
CHECKED BY: A. DAVIS
SUPERVISED BY: D. BLANTON
REVIEWED BY: D. BLANTON
APPROVED BY: D. BLANTON
ISSUED BY: T. JOHNSON

JOHN SEVIER FOSSIL PLANT
TENNESSEE VALLEY AUTHORITY
FOSSIL AND HYDRO ENGINEERING

AUTOCAD R 2000 DATE 01/29/10 41 C 10W507-01 R 0

STANTEC
TASK COMPLETED BY: REV NO.

PLOT FACTOR: XX
W-TVA
C.A.D. DRAWING
DO NOT ALTER MANUALLY

Johnsonville Fossil Plant (JOF)

Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants

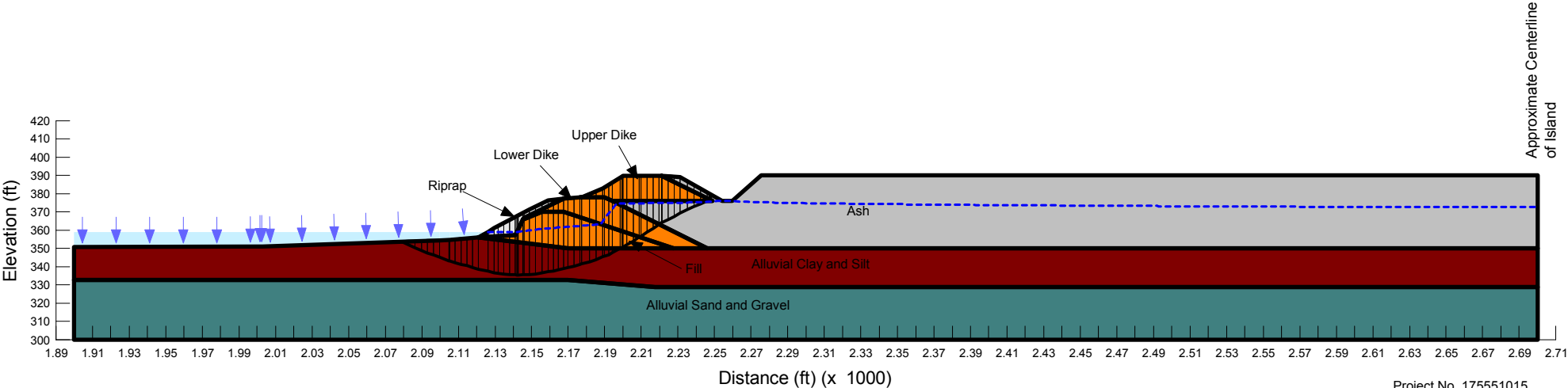


Section K - Ash Disposal Area No. 2
Johnsonville Fossil Plant
New Johnsonville, Tennessee

Note:
The results of analysis shown here are based on available subsurface information,
laboratory test results and approximate soil properties. No warranties can be made
regarding the continuity of subsurface conditions between the borings.

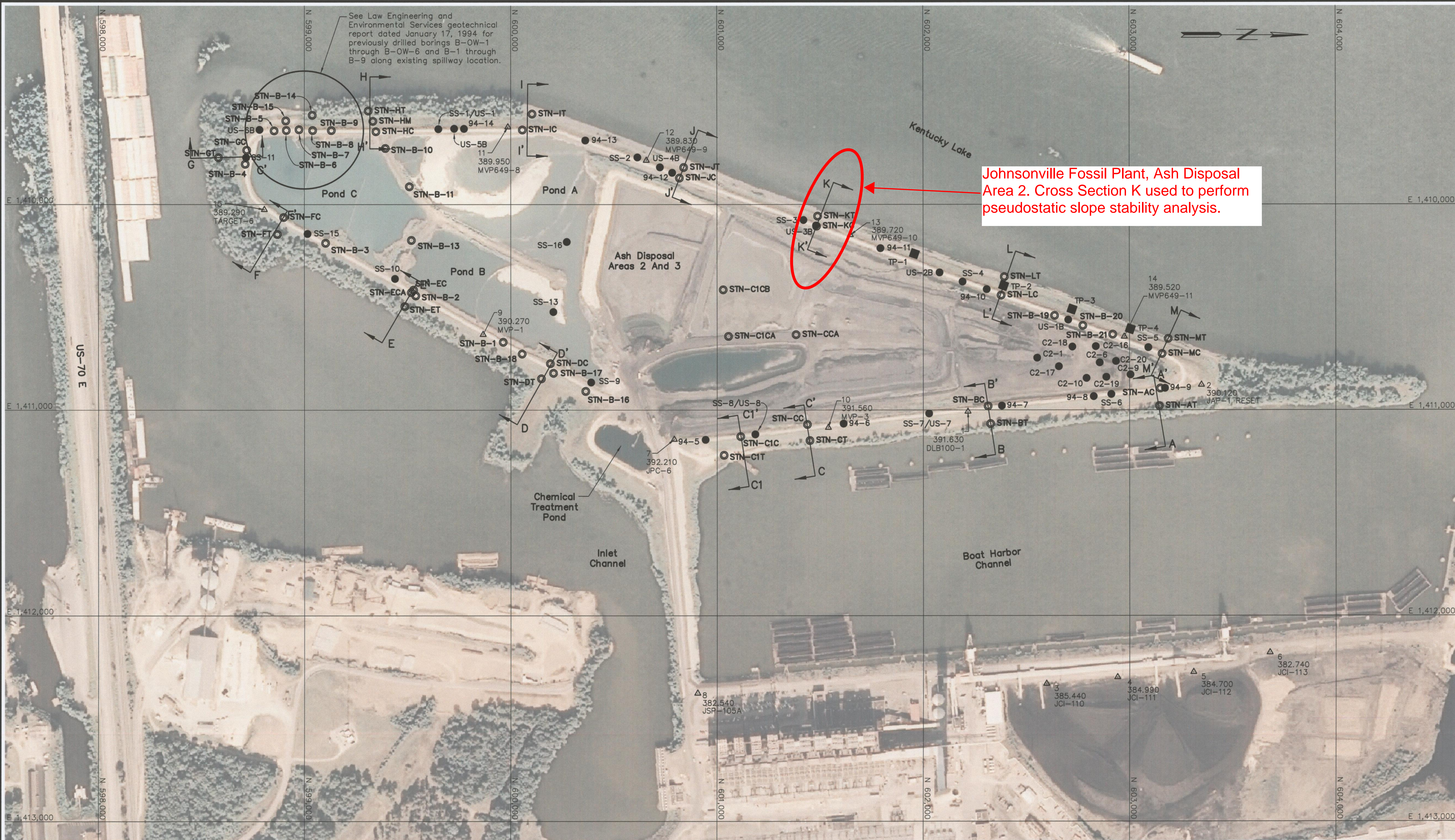
Material Type	Unit Weight (pcf)	Cohesion (psf)	Friction Angle
Upper Dike	125	521	16.2 °
Lower Dike	125	533	20.1 °
Ash	100	0	10 °
Fill	124	630	17.8 °
Alluvial Clay and Silt	124	714	17.8 °
Alluvial Sand and Gravel	120	0	30 °
Riprap	100	0	38 °

Factor of Safety: 1.0
Horizontal Seismic Coefficient $K_h = 0.254\ g$
2500-year Return Period Event



Date of Assessment - 11/22/11

Project No. 175551015



Johnsonville Fossil Plant, Ash Disposal Area 2. Cross Section K used to perform pseudostatic slope stability analysis.

BORING LOCATION TABLE			
BORING	NORTHING	EASTING	ELEV. (FT.)
STN-AC	603,148.82	1,410,894.84	391.4
STN-AT	603,144.12	1,410,980.20	368.4
STN-BC	602,313.93	1,410,981.18	391.5
STN-BT	602,326.17	1,411,067.30	369.8
STN-C1C	601,113.79	1,411,129.20	391.0
STN-C1CA	601,054.92	1,410,641.77	394.0
STN-C1CB	601,029.42	1,410,415.09	398.4
STN-C1T	601,033.28	1,411,220.15	365.5
STN-CC	601,437.52	1,411,070.75	391.6
STN-CCA	601,382.49	1,410,633.59	394.6
STN-CT	601,449.55	1,411,148.76	368.9
STN-DC	600,191.17	1,410,774.31	390.0
STN-DT	600,147.64	1,410,847.53	365.3
STN-EC	599,528.35	1,410,416.19	390.2
STN-ECA	599,517.65	1,410,428.37	390.2
STN-ET	599,486.09	1,410,496.27	363.8
STN-FC	598,898.88	1,410,062.79	389.4
STN-FT	598,868.34	1,410,145.49	362.9
STN-GC	598,719.43	1,409,736.38	389.6
STN-GT	598,582.54	1,409,772.40	360.8
STN-HC	599,345.93	1,409,646.07	389.5
STN-HM	599,331.00	1,409,595.58	377.9
STN-HT	599,308.41	1,409,545.23	363.1
STN-IC	600,055.90	1,409,637.66	389.8
STN-IT	600,103.14	1,409,560.28	368.8
STN-JC	600,817.61	1,409,871.68	389.6
STN-JT	600,838.26	1,409,820.33	378.7
STN-KC	601,482.90	1,410,105.77	389.8
STN-KT	601,488.26	1,410,056.92	377.6
STN-LC	602,377.53	1,410,442.03	389.9
STN-LT	602,392.94	1,410,352.26	366.3
STN-MC	603,157.11	1,410,726.95	390.6
STN-MT	603,187.15	1,410,653.44	365.6
*STN-B-1			390.6
*STN-B-2			390.2
*STN-B-3			390.2
*STN-B-4			389.5
*STN-B-5			389.9
*STN-B-6			389.9
*STN-B-7			390.1
*STN-B-8			389.9
*STN-B-8A			389.9
*STN-B-9			389.7
*STN-B-10			389.1
*STN-B-11			389.6
*STN-B-12	Not Drilled		
*STN-B-13			390.1
*STN-B-14			367.3
*STN-B-15			378.9
*STN-B-16			389.6
*STN-B-17			389.1
*STN-B-18			391.0
*STN-B-19			388.3
*STN-B-20			388.9
*STN-B-21			389.2

*Borings not surveyed and elevations are approximate.

INSPECTION TEST PIT LOCATION TABLE			
BORING	NORTHING	EASTING	ELEV. (FT.)
TP-1	601,957.34	1,410,239.85	-
TP-2	602,389.71	1,410,395.38	-
TP-3	602,721.20	1,410,510.10	-
TP-4	603,003.70	1,410,606.32	-

*Test Pit Locations are approximate.

FOR INFORMATION ONLY
This Record Drawing which has been previously submitted to TVA is provided for Information Only.

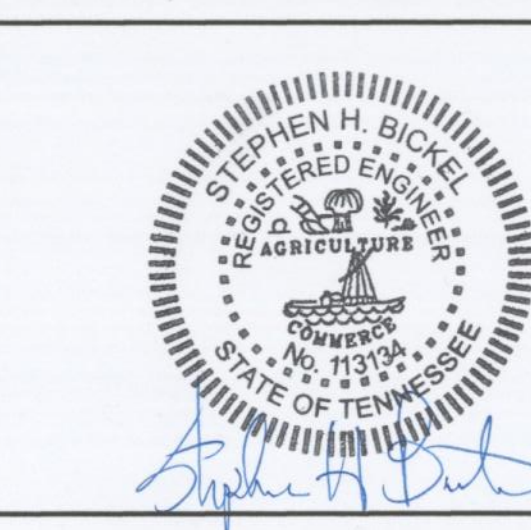
RECORD DRAWING

BORINGS		REFERENCE
B-1 through B-9 and B-OW-1 through B-OW-6		Report of Geotechnical Exploration, Ash Pond Dike, New Johnsonville Plant, New Johnsonville, Tennessee, Law Engineering and Environmental Services, Inc., January 17, 1994.
94-5 through 94-14		Subsurface Exploration Data, TVA Borings at Johnsonville Fossil Plant, Johnsonville, Tennessee, Law Engineering and Environmental Services, Inc., October 11, 1994.
US-1B through US-6B		Johnsonville Steam Plant - Ash Pond - Soil and Foundation Exploration, J.C. McGraw, TVA Construction Services Branch, September 17, 1969.
SS-13, SS-15, SS-16		Johnsonville Groundwater Assessment, TVA Resource Group, Engineering Services, March 1995.
SS-1 through SS-11 and US-1, US-7 and US-8		Johnsonville Steam Plant - Ash Disposal Area No. 2 Dike Raising - Soil Exploration and Testing, G. Farmer, TVA Construction Services Branch, November 22, 1977.
C2-1 through C2-20		Report of Ash Pond Investigation, Johnsonville Fossil Plant, New Johnsonville, Tennessee, MACTEC Engineering and Consulting, December 4, 2003.

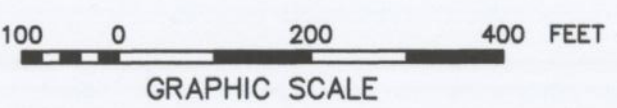
- LEGEND**
- Soil Boring with Undisturbed (Shelby) Tube Samples and/or Standard Penetration Tests
 - Soil Boring from previous explorations by Others
 - Excavated Inspection Test Pit
 - ▲ TVA Survey Monument

- NOTES:**
- Horizontal and vertical locations of 2009 borings provided by T.V.A. Location of previously drilled borings approximated from boring layouts provided in previous reports.
 - The geotechnical information and data furnished herein are not intended as representation or warranties but are furnished for information only. It shall be distinctly understood that the Owner or Engineer will not be responsible for any deduction, interpretation or conclusion drawn therefrom. The information is made available in order that the Contractor may have ready access to the same information available to the Owner and the Engineer and is not part of this contract.

For Supporting Design Calculations see



Stantec Consulting Services Inc.
1901 Nelson Miller Pkwy.
Louisville, Kentucky 40223-2177
Tel. 502.212.5000
Fax 502.212.5055
www.stantec.com



R - - - - -													
R O 01/27/10 PC JM PC SHB SHB SHB TJ - - -													
REV. NO.	DATE	DSGN	DRWN	CHGD	SUPV	RYND	APPD	ISSD	PROJECT	AS CONST	REV NO.		
SCALE: 1"=200'													
EXCEPT AS NOTED													
YARD ASH DISPOSAL AREAS 2 AND 3													
GEOTECHNICAL EXPLORATION BORING LAYOUT													
DESIGNED BY: P. COOPER		DRAWN BY: J. MCKINNEY		CHECKED BY: P. COOPER		SUPERVISED BY: S. BICKEL		REVIEWED BY: S. BICKEL		APPROVED BY: S. BICKEL			
										ISSUED BY: T. JOHNSON			
JOHNSONVILLE FOSSIL PLANT TENNESSEE VALLEY AUTHORITY FOSSIL AND HYDRO ENGINEERING													
AUTOCAD R 2000		DATE 01/27/10		30		C		XXWXXX-01				R 0	

PLOT FACTOR:XX
W_TVA

C.A.D. DRAWING
DO NOT ALTER MANUALLY

Kingston Fossil Plant (KIF)

Pseudostatic Slope Stability Analysis CCP Storage Facilities - Existing Conditions Tennessee Valley Authority Fossil Plants

Section STA. 132+37
Kingston Fossil Plant
Harriman, Tennessee

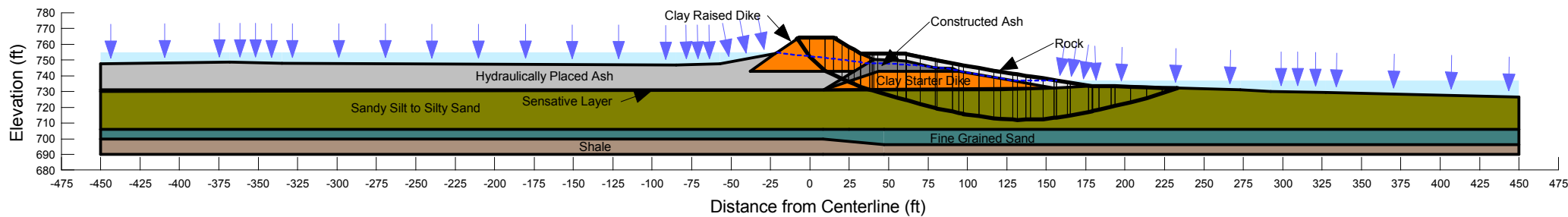


Note:
The results of the analysis shown here are based on available subsurface information, laboratory test results, and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Clay Raised Dike	125 pcf	65 psf	23 °
Constructed Ash	93 pcf	0 psf	30 °
Hydraulically Placed Ash	96 pcf	0 psf	10 °
Sandy Silt to Silty Sand	105 pcf	0 psf	27 °
Fine Grained Sand	122 pcf	0 psf	31 °
Clay Starter Dike	129 pcf	300 psf	26 °
Sensitive Layer	127 pcf	c/p = 0.32	N/A
Rock	110 pcf	0 psf	38 °
Shale	N/A	N/A	N/A

Factor of Safety: 1.0

Horizontal Seismic Coefficient $K_h = 0.115 g$
2500-year Return Period Event



BORING LOCATION TABLE			
Boring No.	Northing	Easting	Elevation (Feet)
STN-2	556804.57	2442329.32	751.20
STN-2B	556801.29	2442323.04	751.00
STN-3	556756.78	2442263.59	763.70
STN-3C	556763.50	2442257.42	763.80
STN-3D	556762.50	2442258.42	763.80
STN-3E	556764.10	2442559.45	763.80
STN-4	556825.56	2442047.01	763.30
STN-5	556811.15	2442365.31	764.90
STN-6	556416.17	2442407.10	763.40
STN-8	556248.52	2442540.30	752.20
STN-8A	556254.98	2442535.17	751.70
STN-9	556233.82	2442499.72	764.80
STN-9A	556234.82	2442499.81	764.80
STN-10	556162.76	2442251.63	765.00
STN-11	556034.83	2442535.05	763.20
STN-12	555873.09	2442622.48	765.10
STN-14	555885.58	2442733.21	753.10
STN-14B	555890.29	2442730.41	753.00
STN-15	555862.69	2442687.52	763.70
STN-15C	555871.50	2442690.88	765.30
STN-15D	555865.72	2442690.78	765.30
STN-15E	555894.24	2442681.34	765.00
STN-16	555501.40	2442725.94	764.50
STN-18	555204.87	2442894.11	751.00
STN-18A	555204.57	2442886.00	751.80
STN-19	555204.68	2442842.94	765.60
STN-19A	555204.68	2442843.94	765.80
STN-20	555168.84	2442866.59	762.90
STN-21	555076.61	2442149.85	765.00
STN-22	554990.27	2441723.40	765.00
STN-23	555020.22	2442857.46	764.70
STN-24	554803.45	2442843.16	765.10
STN-26	554824.86	2442889.00	750.00
STN-26B	554804.57	2442896.12	751.00
STN-27	554801.77	2442850.67	765.10
STN-27A	554800.84	2442840.21	765.00
STN-27B	554806.18	2442840.52	765.00
STN-27C	554807.18	2442840.92	765.00
STN-28	554406.25	2442841.10	764.80
STN-29	554155.15	2442854.72	764.70
STN-31	553954.94	2442758.22	749.50
STN-31A	553960.30	2442764.66	749.70
STN-32	553994.90	2442746.44	764.80
STN-32A	553996.72	2442757.13	764.80
STN-34	553847.51	2442381.24	764.70

BORING LOCATION TABLE			
Boring No.	Northing	Easting	Elevation (Feet)
STN-36	553776.74	2442198.78	751.90
STN-36B	553753.47	2442155.66	751.50
STN-37	553799.90	2442184.40	763.80
STN-37C	553798.81	2442171.01	763.80
STN-37D	553798.26	2442168.95	763.60
STN-37E	553798.26	2442183.40	763.80
STN-38	553730.83	2441988.70	764.10
STN-41	553583.10	2441510.71	752.70
STN-41A	553584.59	2441512.86	751.80
STN-42	553623.48	2441513.69	764.70
STN-42A	553624.48	2441512.69	764.70
STN-43	554004.68	2441548.50	765.90
STN-43A	554005.71	2441549.61	765.90
STN-45	553740.50	2441308.40	763.90
STN-47	553747.39	2441146.83	753.40
STN-47B	553742.43	2441143.28	753.80
STN-48	553773.29	2441154.53	765.30
STN-48C	553770.51	2441164.28	765.30
STN-49	553921.85	2441024.39	763.10
STN-50	553631.96	2440498.85	741.60
STN-50A	553632.98	2440497.85	741.60
STN-51	553696.02	2440548.46	750.40
STN-52	553992.98	2440817.40	753.20
STN-53	554011.12	2440802.46	763.90
STN-53A	554012.14	2440804.05	763.90
STN-54	555263.94	2441476.12	765.00
STN-55	554943.73	2442287.49	764.10
STN-56	554555.61	2441998.50	765.80
STN-59	556075.53	2442603.08	752.20
STN-60	555886.92	2442663.31	752.50
STN-61	555513.59	2442792.80	752.50
STN-62	555020.69	2442907.23	749.80
STN-63	554822.75	2442910.57	750.00
STN-64	554411.29	2442811.08	749.40
STN-65	554147.51	2442915.09	748.80
STN-66	553888.83	2442564.24	750.90
STN-69	553607.58	2441718.01	752.30
STN-71	553840.20	2440981.04	752.00
STN-72	556243.56	2442498.40	765.70
STN-73	556264.59	2442544.34	763.90
STN-74	555189.21	2442848.63	766.20
STN-74A	555184.59	2442849.41	766.20
STN-75	555194.95	2442895.83	753.40
STN-76	553923.59	2442380.21	762.00
STN-77	553847.01	2442405.85	754.90

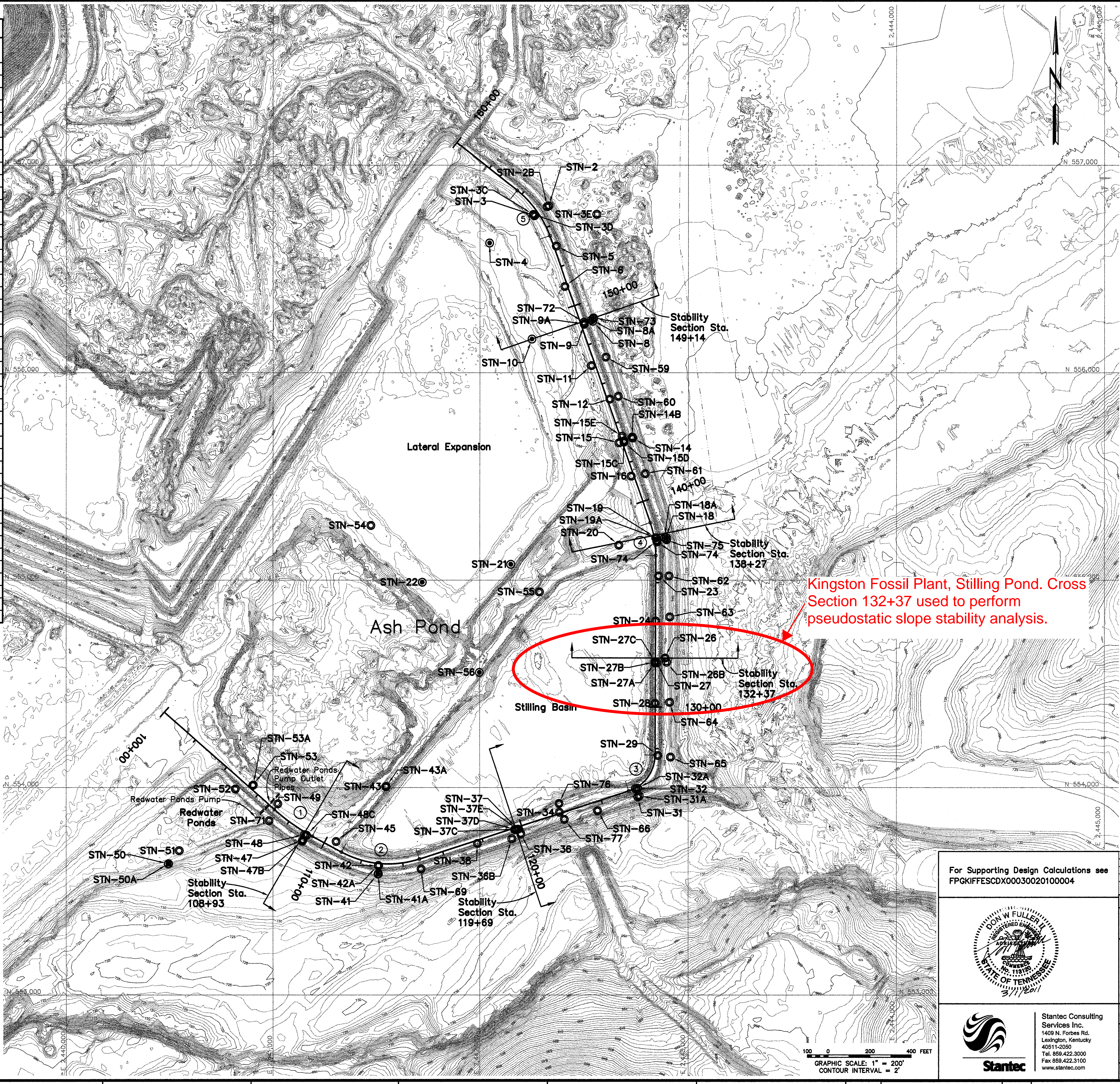


TABLE OF BASELINE COORDINATES			
Station	Baseline	Northing	Easting
100+00.00	Begin Project Baseline	554,363.86	2,440,477.77
160+00.00	End Project Baseline	557,109.43	2,441,888.38

BASELINE CURVE DATA

①	②
P.I. Sta. = 108+20.74 Northing = 553,819.18 Easting = 2,441,091.71 $\Delta = 13'20.06''$ R = 974.68' T = 113.94' L = 226.85' E = 6.64' P.C. Sta. = 107+06.80 P.T. Sta. = 109+33.65	P.I. Sta. = 112+99.93 Northing = 553,591.92 Easting = 2,441,514.77 $\Delta = 46'19.10''$ R = 479.84' T = 205.26' L = 387.92' E = 42.06' P.C. Sta. = 110+94.67 P.T. Sta. = 114+82.59
③	④
P.I. Sta. = 126+71.85 Northing = 554,024.61 Easting = 2,442,840.47 $\Delta = 71'45.50''$ R = 182.94' T = 132.33' L = 229.13' E = 42.85' P.C. Sta. = 125+39.52 P.T. Sta. = 127+68.65	P.I. Sta. = 138+12.12 Northing = 555,200.41 Easting = 2,442,843.77 $\Delta = 19'21.32''$ R = 479.84' T = 81.84' L = 162.13' E = 6.93' P.C. Sta. = 137+30.28 P.T. Sta. = 138+92.41
⑤	P.I. Sta. = 154+96.30 Northing = 556,792.40 Easting = 2,442,289.43 $\Delta = 32'28.34''$ R = 479.84' T = 139.75' L = 271.98' E = 19.94' P.C. Sta. = 153+56.55 P.T. Sta. = 156+28.53

LEGEND

- Soil Boring with Undisturbed (Shelby) Tube Samples and/or Standard Penetration Tests
- ⊙ Soil Boring with Undisturbed (Shelby) Tube Samples and/or Standard Penetration Tests and Rock Core
- Previously Drilled by AECOM
- ⊙ Cone Penetration Test

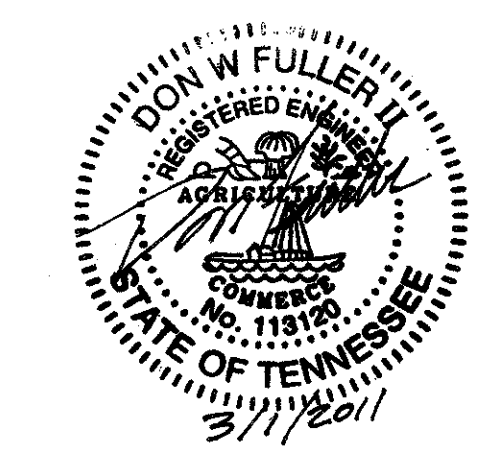
NOTES

- Topographic mapping was generated from a LIDAR survey performed on January 8, 2009 by Tuck Mapping Solutions, Inc. This plan was developed for discussion purposes only and is not to be used for construction.
- The geotechnical information and data furnished herein are not intended as representation or warranties but are furnished for information only. It shall be distinctly understood that the Owner or Engineer will not be responsible for any deduction, interpretation or conclusion drawn therefrom. The information is made available in order that the Contractor may have ready access to the same information available to the Owner and the Engineer and is not part of this contract.

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RECORD DRAWING

For Supporting Design Calculations see
FPKIFFESCDX00030020100004



Stantec Consulting Services Inc.
1409 N. Forbes Rd.
Lexington, Kentucky 40511-2050
Tel. 859.422.3000
Fax 859.422.3100
www.stantec.com

DATE	03/01/11	DESIGN	DRWN	CHKD	SUPV	RVD	APPD	ISSD	PROJECT	AS CONST	EXCEPT AS NOTED
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SCALE: 1"=200'

YARD
DIKE C
BUTTRESS DESIGN
BORING PLAN AND BASELINE LAYOUT

DESIGNED BY	DRWN BY	CHECKED BY	SUPERVISED BY	REVIEWED BY	APPROVED BY	ISSUED BY
J.D. PINEAU	T. JOHNSON	T. GRILLY	D.W. FULLER	-	M.S. TURNBOW	J.C. KAMMEYER

KINGSTON FOSSIL PLANT
TENNESSEE VALLEY AUTHORITY
FOSSIL AND HYDRO ENGINEERING

AUTOCAD R 2000	DATE	03/01/11	36	C	10W287-01	R 0
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GRAPHIC SCALE: 1" = 200'
CONTOUR INTERVAL = 2'

STANTEC	0
TASK COMPLETED BY:	REV NO.

PLOT FACTOR:200
W_TVA

C.A.D. DRAWING
DO NOT ALTER MANUALLY

Paradise Fossil Plant (PAF)



**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**

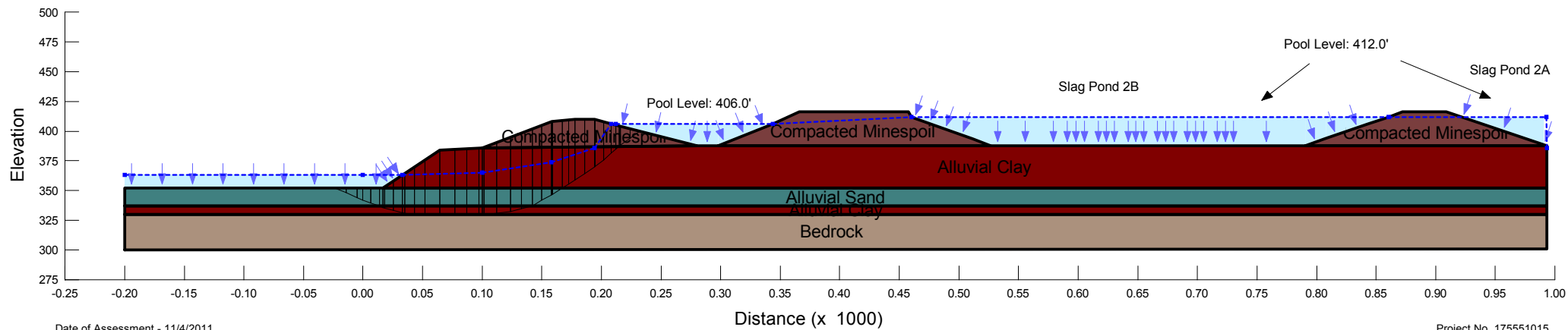
**Section - Slag Pond 2B
Paradise Fossil Plant
Drakesboro, Kentucky**

Note:
The results of analysis shown here are based on available subsurface information,
laboratory test results and approximate soil properties. No warranties can be made
regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Compacted Minespoil	125 pcf	120 psf	22.6 °
Alluvial Clay	125 pcf	975 psf	12.9 °
Alluvial Sand	120 pcf	1000 psf	19.2 °

Factor of Safety: 1.1

Horizontal Sismic Coefficient $K_h = 0.157 g$
2500 year Return Period Event



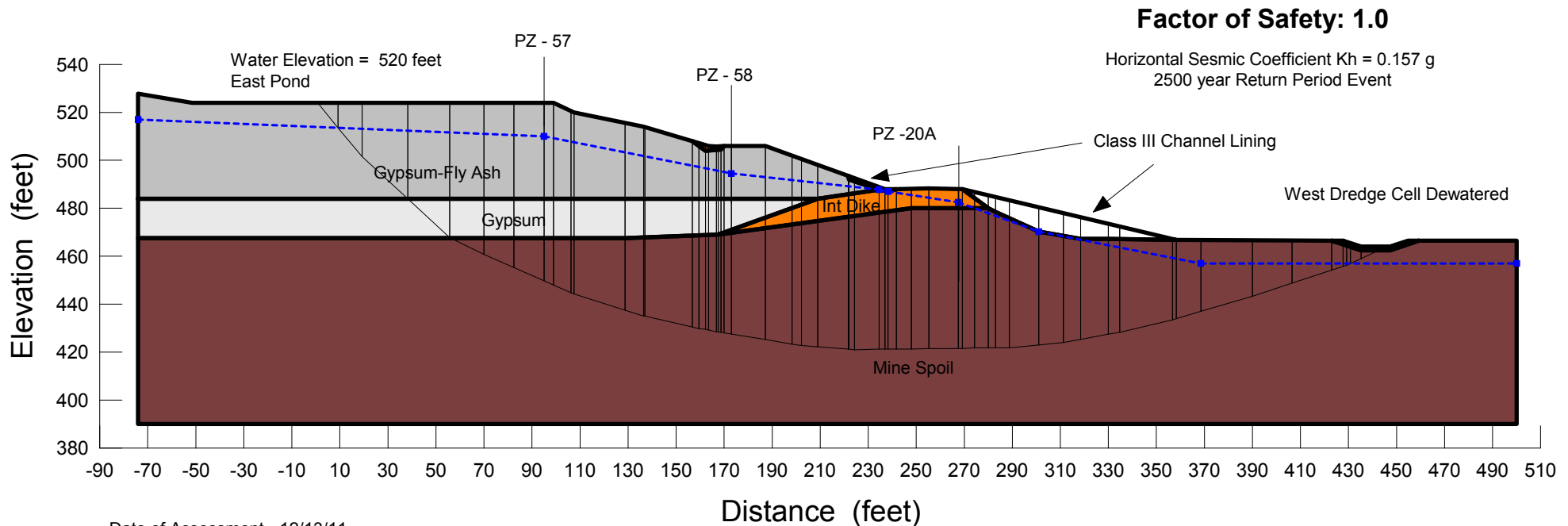
Pseudostatic Slope Stability Analysis CCP Storage Facilities - Existing Conditions Tennessee Valley Authority Fossil Plants

Section G - Scrubber Sludge Complex Paradise Fossil Plant Drakesboro, Kentucky



Note:
The results of analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Gypsum	120	0	35
Gypsum-Fly Ash	115	0	35
Mine Spoil	135	400	18
Compacted Mine Spoil	135	120	22.6
Int Dike	110	120	22.6
Class III Channel	110	0	40



Date of Assessment - 12/13/11

Project No. 175551015

**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**

**Section A - Peabody Ash Pond
Paradise Fossil Plant
Drakesboro, Kentucky**

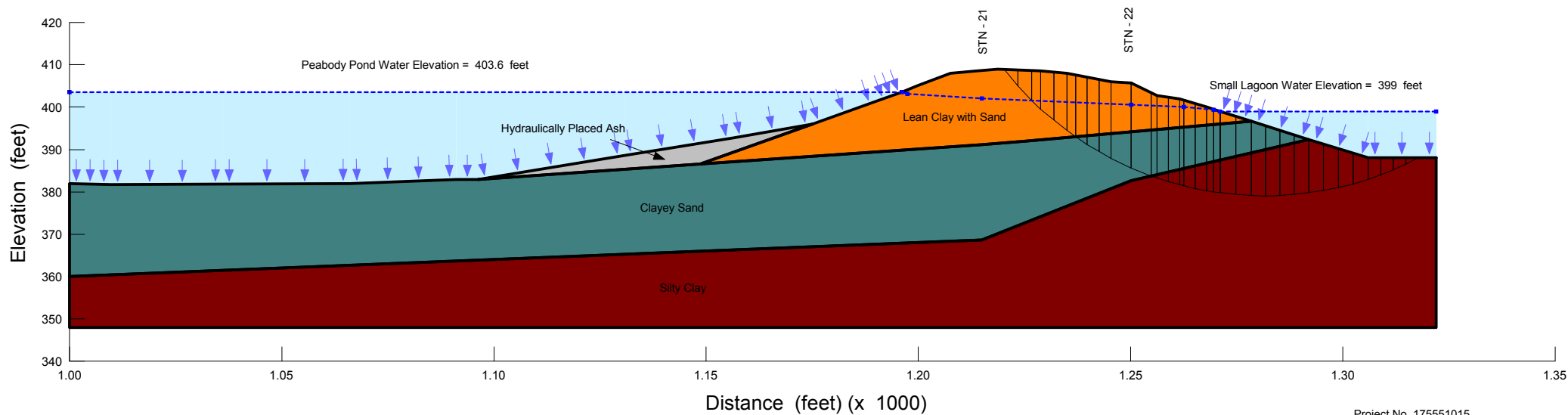


Material Type	Unit Weight	Cohesion	Friction Angle
Lean Clay with Sand	139 pcf	0 psf	25 °
Hydraulically Placed Ash	107 pcf	100 psf	18.4 °
Clayey Sand	133 pcf	120 psf	21 °
Silty Clay	129 pcf	120 psf	20 °

Note:
The results of analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

Factor of Safety: 1.0

Horizontal Seismic Coefficient $K_h = 0.157 g$
2500 year Return Period Event



KEYNOTES

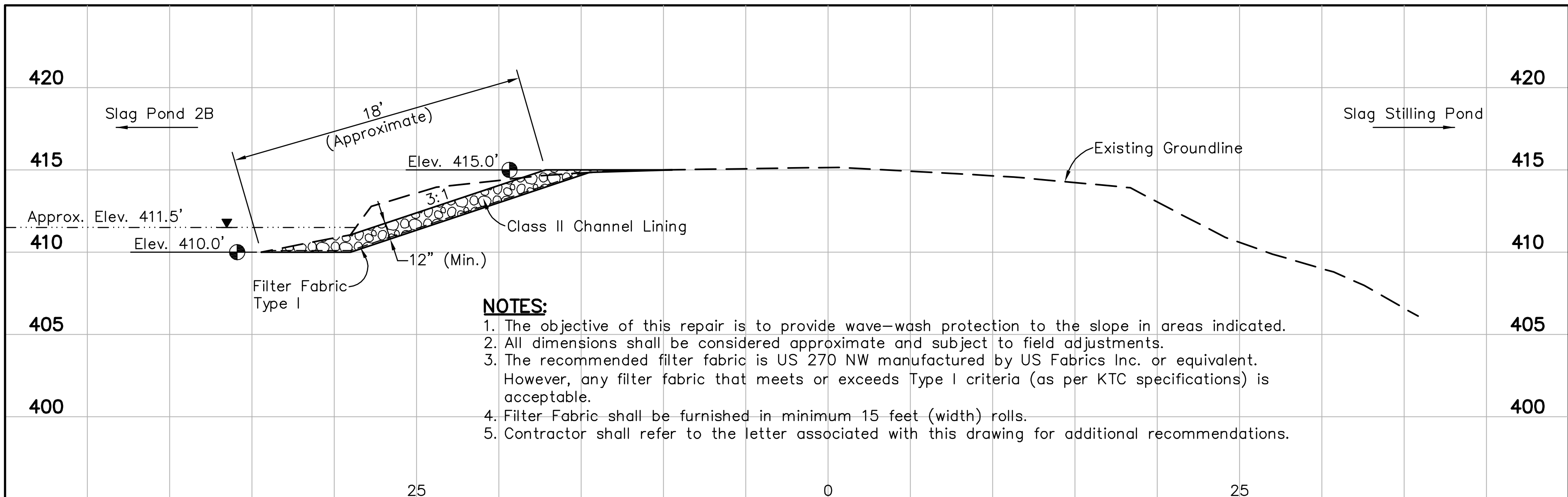
- Existing 48" Pipe (2).
- Existing 60" Pipe.
- Begin Slope Improvements (Both sides)
N 341,765.19
E 1,641,511.25
- End Slope Improvements (Both sides)
N 341,099.61
E 1,641,746.04
- Begin Class II Channel Lining at Concrete Flume
(Pool side, Slag Pond 2B)
N 341,499.29
E 1,642,107.18
- End Class II Channel Lining
(Pool side, Slag Pond 2B)
N 341,915.20
E 1,641,500.55
- Begin Class II Channel Lining
(Pool side, Slag Pond 2B)
N 340,948.45
E 1,642,306.10
- End Class II Channel Lining at concrete flume
(Pool side, Slag Pond 2B)
N 341,482.01
E 1,642,120.86
- Begin Class III Channel Lining
(Pool side, Slag Stilling Pond)
N 341,404.43
E 1,642,212.95
- End Class III Channel Lining at Concrete Flume
(Pool side, Slag Stilling Pond)
N 341,487.93
E 1,642,166.15
- Begin Class III Channel Lining at Concrete Flume
(Pool side, Slag Stilling Pond)
N 341,509.20
E 1,642,154.45
- End Class III Channel Lining
(Pool side, Slag Stilling Pond)
N 341,445.91
E 1,642,343.78

NOTES:

- Horizontal datum of coordinates shown is NAD27 Kentucky State Plane South Zone (feet).
- Vertical datum is NGVD29.



PLAN - COAL YARD SLAG POND 2B AND SLAG STILLING POND
SCALE: 1"=100'



NOTES:

- The objective of this repair is to provide wave-wash protection to the slope in areas indicated.
- All dimensions shall be considered approximate and subject to field adjustments.
- The recommended filter fabric is US 270 NW manufactured by US Fabrics Inc. or equivalent. However, any filter fabric that meets or exceeds Type I criteria (as per KTC specifications) is acceptable.
- Filter Fabric shall be furnished in minimum 15 feet (width) rolls.
- Contractor shall refer to the letter associated with this drawing for additional recommendations.

1 TYPICAL CROSS SECTION A - SLOPE IMPROVEMENT
WITH CLASS II CHANNEL LINING (SLAG POND 2B)
SCALE: 1"=5'

Paradise Fossil Plant, Slag Ponds 2A and 2B. Approximate Location of Cross Section (Typical) used to perform pseudostatic slope stability analysis.

NOTE:

Slope improvements involving cut within 10 feet of structures shall be avoided. The slope may be improved through the placement of Class II Channel Lining where feasible under the direction of TVA PAF plant personnel.

EXCESS MATERIAL DISPOSAL NOTE:

Excess material produced by improvements shown hereon shall be disposed of at a disposal facility on-site as directed by the TVA PAF plant personnel.

UTILITY NOTE:

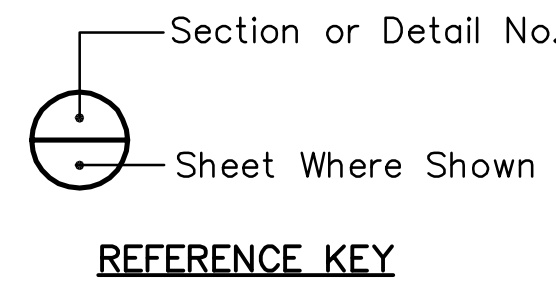
The location, sizes and types of any utilities or substructures shown hereon shall be considered approximate. Stantec does not warranty or guarantee that the utility information shown hereon is accurate or complete. Any Contractor, Owner or designer using information shown is hereby forewarned that any excavation upon this site may result in the discovery of additional underground utilities not shown hereon.

Prior to any design or construction in the vicinity of any improvements shown hereon, the Contractor shall coordinate with TVA PAF Plant personnel to determine the location of all utilities within the area. The Contractor shall be responsible for locating all utilities, making arrangements regarding relocation and/or protection as necessary, and maintaining utility service throughout the course of the work.

Ponds located within the fenced portions of the Plant are situated in areas with numerous above-ground and underground utilities.

FOR INFORMATION ONLY

This Drawing which has been previously submitted to TVA is provided for Information Only.



-											
R 0	12/21/09	MMM	BFS	JOK	ELC	ELC	ELC	TJ	-		
ISSUED FOR CONSTRUCTION											
REV. NO.	DATE	DSN	DRWN	CHD	SUPV	RVID	APPO	ISSD	PROJECT ID	AS CONST	REV NO
SCALE: AS SHOWN									EXCEPT AS NOTED		
YARD											
-											
COAL YARD SLAG PONDS 2A, 2B AND SLAG STILLING POND - SLOPE IMPROVEMENTS-PLAN AND SECTIONS											
DESIGNED BY:	DRAWN BY:	CHECKED BY:	SUPERVISED BY:	REVIEWED BY:	APPROVED BY:	ISSUED BY:					
M. MEEHAN	B. SAMS	J. KEELING	E. CAUDILL	E. CAUDILL	E. CAUDILL	T. JOHNSON					
PARADISE FOSSIL PLANT TENNESSEE VALLEY AUTHORITY FOSSIL AND HYDRO ENGINEERING											
AUTOCAD R 2000		DATE	64	C	10W709-01				R 0		
		12/21/09									



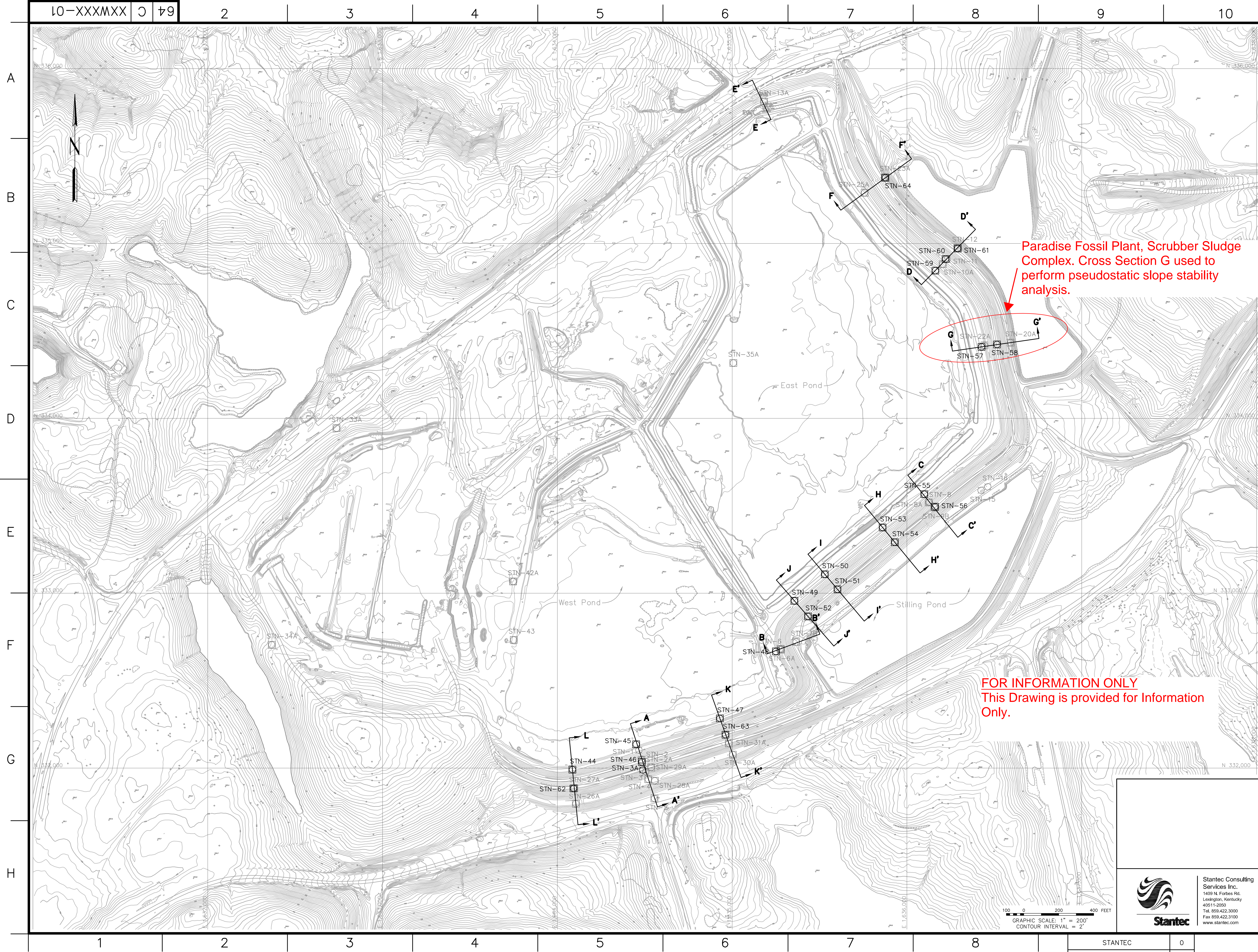
Stantec Consulting Services Inc.
1409 N. Forbes Rd.
Lexington, Kentucky 40511-2050
Tel 859.422.3000
Fax 859.422.3100
www.stantec.com

ISSUED FOR CONSTRUCTION

STANTEC	0
TASK COMPLETED BY:	REV NO.

PLOT FACTOR:XX
W_TVA

C.A.D. DRAWING
DO NOT ALTER MANUALLY



NOTES:

1. THE TOPOGRAPHIC BASEMAP SHOWN ON THIS DRAWING WAS DEVELOPED BY STANTEC USING AN ELECTRONIC DRAWING PROVIDED BY TVA IN MARCH, 2010. THE INFORMATION IS BELIEVED TO BE APPROXIMATE AND SHOULD NOT BE USED FOR CONSTRUCTION PURPOSES.
2. THE GEOTECHNICAL INFORMATION AND DATA FURNISHED HEREIN ARE NOT INTENDED AS REPRESENTATION OR WARRANTIES BUT ARE FURNISHED FOR INFORMATION ONLY. IT SHALL BE DISTINCTLY UNDERSTOOD THAT THE OWNER OR ENGINEER WILL NOT BE RESPONSIBLE FOR ANY DEDUCTION, INTERPRETATION OR CONCLUSION DRAWN THERE FROM. THE INFORMATION IS MADE AVAILABLE IN ORDER THAT THE CONTRACTOR MAY HAVE READY ACCESS TO THE SAME INFORMATION AVAILABLE TO THE OWNER AND THE ENGINEER AND IS NOT PART OF THIS CONTRACT.

INSTRUMENTATION LOCATION TABLE

INSTRUMENT	NORTHING	EASTING	GROUND SURFACE ELEVATION (FEET)	PIEZOMETER TIP ELEVATION (FEET)
STN-1*	332,102.89	1,634,466.78	509.9	NA
STN-2	332,046.91	1,634,482.61	494.3	437.1
STN-2A	332,046.91	1,634,482.61	494.3	468.9
STN-3*	331,980.02	1,634,505.77	483.5	NA
STN-3A*	331,989.56	1,634,489.16	488.8	NA
STN-4	331,937.06	1,634,520.16	467.6	420.7
STN-5	331,826.79	1,634,555.94	452.0	399.3
STN-6	332,682.25	1,635,277.08	510.9	413.1
STN-6A	332,677.36	1,635,279.66	511.0	472.6
STN-7B	332,721.84	1,635,366.07	486.8	474.4
STN-8*	333,522.45	1,634,124.32	510.2	510.2
STN-8A	333,519.21	1,636,124.51	510.2	452.3
STN-9B	333,490.39	1,636,156.12	497.5	455.5
STN-10A	334,881.85	1,636,204.33	514.0	465.7
STN-11*	334,909.50	1,636,230.61	504.5	NA
STN-12	334,974.78	1,636,288.23	494.4	464.4
STN-13A	335,811.89	1,635,182.62	525.1	504.0
STN-15*	333,588.73	1,636,423.97	480.9	NA
STN-16*	333,609.25	1,636,462.16	481.1	NA
STN-20A	334,453.28	1,636,592.28	489.7	470.7
STN-22A	334,417.84	1,636,443.36	510.4	490.4
STN-23A	335,379.92	1,635,873.53	501.8	470.5
STN-25A	335,289.98	1,635,757.90	523.3	489.0
STN-26A	331,796.20	1,634,106.29	471.4	454.1
STN-27A	331,884.74	1,634,097.38	492.2	470.3
STN-28A	331,927.41	1,634,557.82	492.2	454.2
STN-29A	332,004.37	1,634,537.00	480.1	470.9
STN-30A	332,077.28	1,635,004.12	470.5	455.6
STN-31A	332,141.29	1,634,981.22	487.0	468.9
STN-33A	333,944.10	1,632,377.00	527.2	478.3
STN-34	332,705.00	1,632,367.00	520.9	485.6
STN-35A	334,316.70	1,635,007.00	520.4	496.6
STN-42	333,067.40	1,633,747.00	515.5	500.7
STN-42A	333,067.40	1,633,747.00	515.7	500.7
STN-43	332,733.20	1,633,751.00	513.5	487.9
STN-44	331,991.72	1,634,086.60	510.0	498.1
STN-45	332,136.33	1,634,450.96	517.0	501.0
STN-46	332,034.92	1,634,483.72	496.1	486.1
STN-47	332,284.56	1,634,929.95	516.5	490.5
STN-48	332,667.56	1,635,250.39	519.9	485.1
STN-49	332,956.89	1,635,355.31	518.3	501.0
STN-50	333,108.69	1,635,530.13	524.0	502.9
STN-51	333,022.54	1,635,602.72	498.9	486.0
STN-52	332,867.62	1,635,433.99	495.9	481.5
STN-53	333,376.37	1,635,860.86	523.5	504.1
STN-54	333,291.99	1,635,930.41	500.6	487.1
STN-55	333,566.60	1,636,099.43	523.7	514.7
STN-56	333,494.76	1,636,161.26	500.0	482.0
STN-57	334,409.01	1,636,426.68	524.9	511.9
STN-58	334,422.92	1,636,515.29	506.7	480.1
STN-59	334,845.21	1,636,162.73	526.6	512.0
STN-60	334,911.36	1,636,220.95	508.0	480.7
STN-61	334,971.36	1,636,290.43	494.4	485.4
STN-62	331,883.99	1,634,092.50	491.9	480.1
STN-63	332,191.18	1,634,962.04	494.5	487.0
STN-64	335,375.88	1,635,876.57	502.0	485.4

* DENOTES SLOPE INCLINOMETER

LEGEND

- ☒ EXISTING PIEZOMETER (INSTALLED SEPT. 2010)
☒ EXISTING SLOPE INCLINOMETER (INSTALLED SEPT. 2010)
☐ EXISTING PIEZOMETER (PREVIOUSLY INSTALLED)
☐ EXISTING SLOPE INCLINOMETER (PREVIOUSLY INSTALLED)

[illegible]

SCALE: 1"=200'

EXCEPT AS NOTED

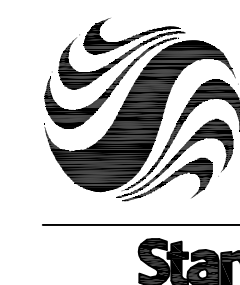
YARD
SCRUBBER SLUDGE COMPLEX

GEOTECHNICAL EXPLORATION INSTRUMENTATION PLAN

DESIGNED BY:	DRAWN BY:	CHECKED BY:	SUPERVISED BY:	REVIEWED BY:	APPROVED BY:	ISSUED BY:
—	—	—	—	—	—	—

PARADISE FOSSIL PLANT
TENNESSEE VALLEY AUTHORITY
FOSSIL AND HYDRO ENGINEERING

AUTOCAD R 2000	DATE -	64	C	XXWXXX-01	R 0
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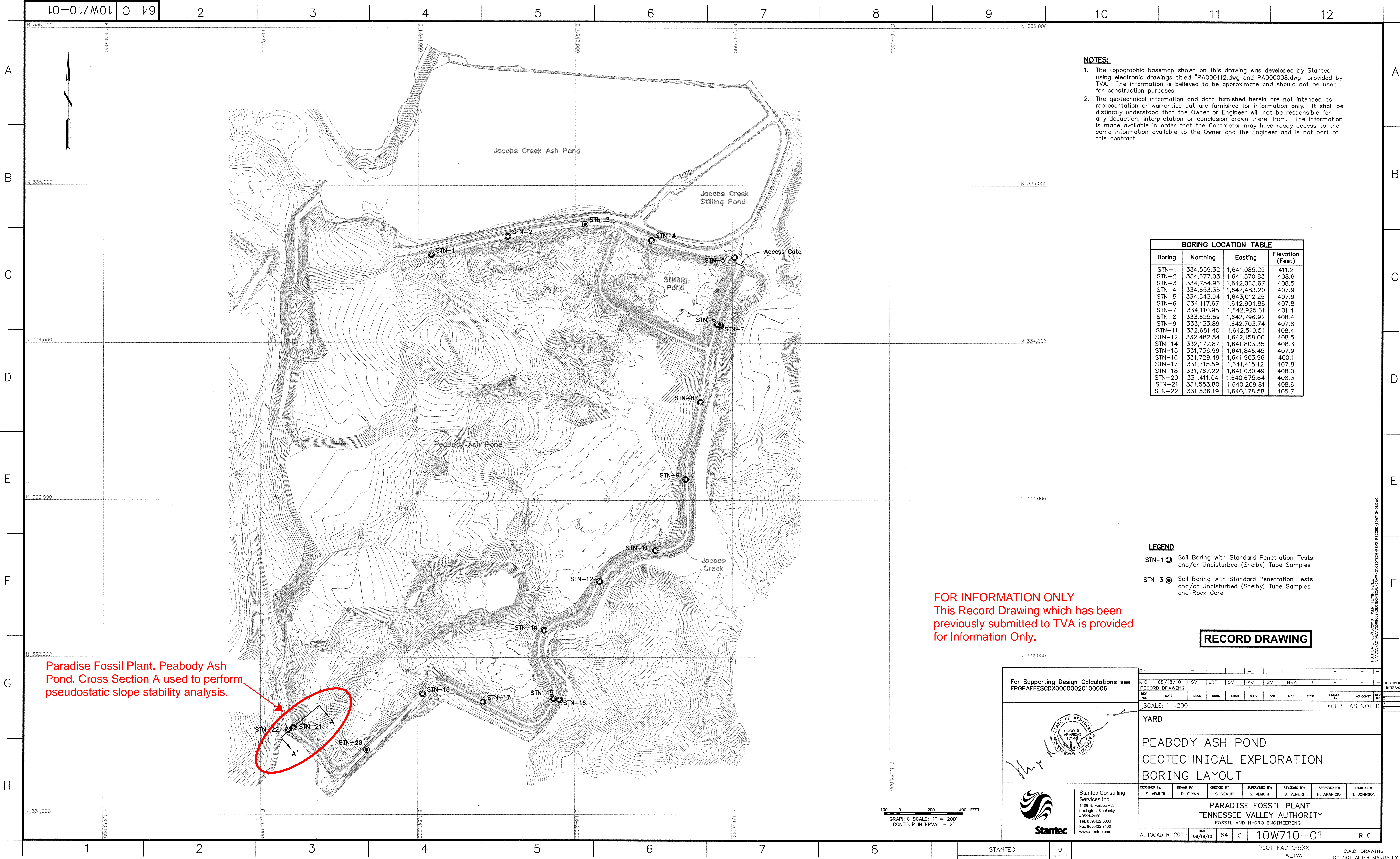
**Stantec Consulting
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1409 N. Forbes Rd.
Lexington, Kentucky
40511-2050
Tel. 859.422.3000
Fax 859.422.3100
www.stantec.com

00 0 200 400 FEET
GRAPHIC SCALE: 1" = 200'

STANTEC	0
TASK COMPLETED BY:	REV NO.

PLOT FACTOR:200
W_TVA

C.A.D. DRAWING
DO NOT ALTER MANUALLY



Paradise Fossil Plant, Peabody Ash Pond. Cross Section A used to perform pseudostatic slope stability analysis.

- NOTES:**
1. The topographic basemap shown on this drawing was developed by Stantec using electronic drawings titled "PA000112.dwg and PA000008.dwg" provided by TVA. The information is believed to be approximate and should not be used for construction purposes.
 2. The geotechnical information and data furnished herein are not intended as representation or warranties but are furnished for information only. It shall be distinctly understood that the Owner or Engineer will not be responsible for any deduction, interpretation or conclusion drawn there-from. The information is made available in order that the Contractor may have ready access to the same information available to the Owner and the Engineer and is not part of this contract.

BORING LOCATION TABLE			
Boring	Northing	Easting	Elevation (Feet)
STN-1	334,559.32	1,641,085.25	411.2
STN-2	334,677.03	1,641,570.83	408.6
STN-3	334,754.96	1,642,063.67	408.5
STN-4	334,653.35	1,642,483.20	407.9
STN-5	334,543.94	1,643,012.25	407.9
STN-6	334,117.67	1,642,904.88	407.8
STN-7	334,110.95	1,642,925.61	401.4
STN-8	333,625.59	1,642,796.92	408.4
STN-9	333,133.89	1,642,703.74	407.8
STN-11	332,681.40	1,642,510.51	408.4
STN-12	332,482.84	1,642,158.00	408.5
STN-14	332,172.87	1,641,803.35	408.3
STN-15	331,736.99	1,641,846.45	407.9
STN-16	331,729.49	1,641,903.96	400.1
STN-17	331,715.59	1,641,415.12	407.8
STN-18	331,767.22	1,641,030.49	408.0
STN-20	331,411.04	1,640,675.64	408.3
STN-21	331,553.80	1,640,209.81	408.6
STN-22	331,536.19	1,640,178.58	405.7

- LEGEND**
- STN-1 Soil Boring with Standard Penetration Tests and/or Undisturbed (Shelby) Tube Samples
- STN-3 Soil Boring with Standard Penetration Tests and/or Undisturbed (Shelby) Tube Samples and Rock Core

FOR INFORMATION ONLY
This Record Drawing which has been previously submitted to TVA is provided for Information Only.

RECORD DRAWING

For Supporting Design Calculations see FPGPAFFESCDX00000020100006		<table><tr><td>R</td><td>0</td><td>08/18/10</td><td>SV</td><td>JRF</td><td>SV</td><td>SV</td><td>SV</td><td>HRA</td><td>TJ</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td></tr><tr><td>RECORD DRAWING</td><td>DATE</td><td>DSGN</td><td>DRWN</td><td>CHKD</td><td>SUPV</td><td>RWMD</td><td>APPD</td><td>ISSD</td><td>PROJECT</td><td>AS CONST</td><td>REV</td><td>NO.</td><td>DATE</td><td>BY</td><td>CHKD</td><td>SUPV</td><td>RWMD</td><td>APPD</td><td>ISSD</td></tr></table>										R	0	08/18/10	SV	JRF	SV	SV	SV	HRA	TJ	-	-	-	-	-	-	-	-	-	-	RECORD DRAWING	DATE	DSGN	DRWN	CHKD	SUPV	RWMD	APPD	ISSD	PROJECT	AS CONST	REV	NO.	DATE	BY	CHKD	SUPV	RWMD	APPD	ISSD
R	0	08/18/10	SV	JRF	SV	SV	SV	HRA	TJ	-	-	-	-	-	-	-	-	-	-																																
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		SCALE: 1"=200' EXCEPT AS NOTED																																																	
		YARD																																																	
		PEABODY ASH POND GEOTECHNICAL EXPLORATION BORING LAYOUT																																																	
		<table><tr><td>DESIGNED BY:</td><td>DRAWN BY:</td><td>CHECKED BY:</td><td>SUPERVISED BY:</td><td>REVIEWED BY:</td><td>APPROVED BY:</td><td>ISSUED BY:</td></tr><tr><td>S. VEMURI</td><td>R. FLTYN</td><td>S. VEMURI</td><td>S. VEMURI</td><td>S. VEMURI</td><td>H. APARICIO</td><td>T. JOHNSON</td></tr></table>																		DESIGNED BY:	DRAWN BY:	CHECKED BY:	SUPERVISED BY:	REVIEWED BY:	APPROVED BY:	ISSUED BY:	S. VEMURI	R. FLTYN	S. VEMURI	S. VEMURI	S. VEMURI	H. APARICIO	T. JOHNSON																		
DESIGNED BY:	DRAWN BY:	CHECKED BY:	SUPERVISED BY:	REVIEWED BY:	APPROVED BY:	ISSUED BY:																																													
S. VEMURI	R. FLTYN	S. VEMURI	S. VEMURI	S. VEMURI	H. APARICIO	T. JOHNSON																																													
		PARADISE FOSSIL PLANT TENNESSEE VALLEY AUTHORITY FOSSIL AND HYDRO ENGINEERING																																																	
		AUTOCAD R 2000 DATE: 08/18/10 64 C 10W710-01 R 0																																																	

GRAPHIC SCALE: 1" = 200'
CONTOUR INTERVAL = 2'

Widows Creek Fossil Plant (WCF)



**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**

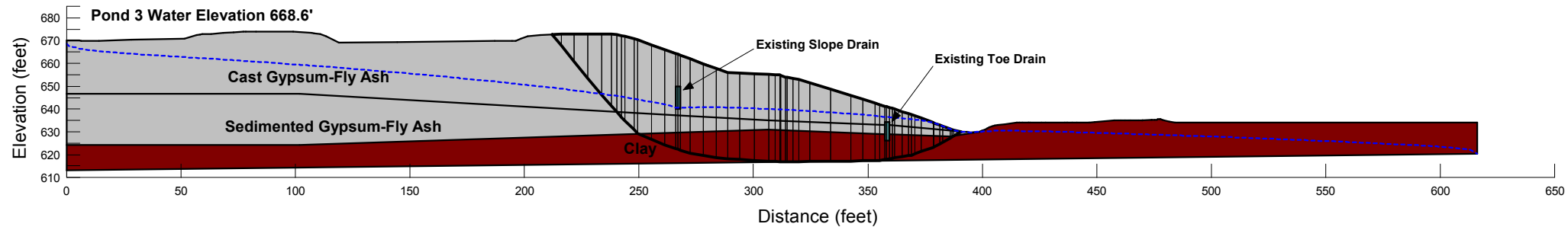
**Section F - Gypsum Stack
Widows Creek Fossil Plant
Stevenson, Alabama**

Note:
The results of analysis shown here are based on available subsurface information,
laboratory test results and approximate soil properties. No warranties can be made
regarding the continuity of subsurface conditions between the borings.

Material Type	Unit Weight	Cohesion	Friction Angle
Cast Gypsum-Fly Ash	113 pcf	0 psf	40 °
Sedimented Gypsum-Fly Ash	112 pcf	0 psf	41 °
Sand Drains	110 pcf	0 psf	33 °
Clay	123 pcf	650 psf	15.7 °

Factor of Safety: 1.5

Horizontal Sismic Coefficient $K_h = 0.1$ g
2500 year Return Period Event



**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**

**Section D - Dredge Cell (Old Scrubber Sludge Pond)
Widows Creek Fossil Plant
Stevenson, Alabama**

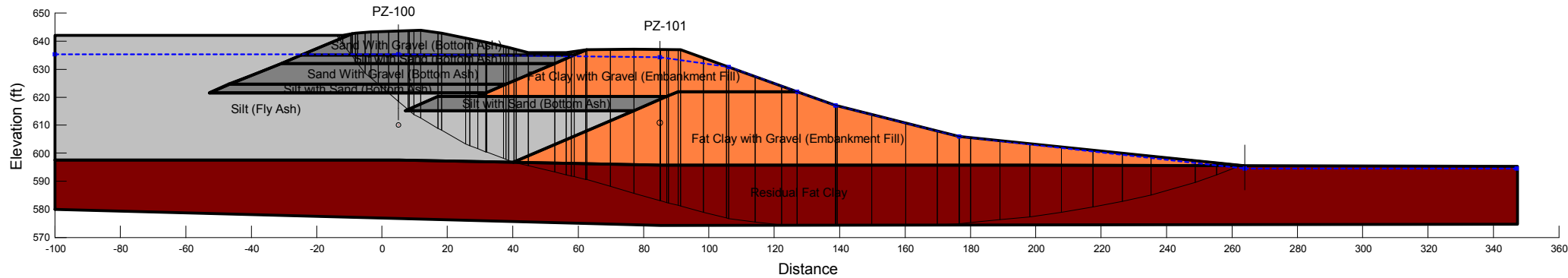


Material Type	Unit Weight	Cohesion	Friction Angle
Residual Fat Clay	125 pcf	650 psf	15.7 °
Silt with Sand (Bottom Ash)	112 pcf	0 psf	33 °
Silt (Fly Ash)	112 pcf	0 psf	21.8 °
Sand With Gravel (Bottom Ash)	119 pcf	0 psf	33 °
Fat Clay with Gravel (Embankment Fill)	125 pcf	1375 psf	14.2 °

Note:
The results of analysis shown here are based on available subsurface information,
laboratory test results and approximate soil properties. No warranties can be made
regarding the continuity of subsurface conditions between the borings.

Factor of Safety: 1.1

Horizontal Sismic Coefficient $K_h = 0.1 \text{ g}$
2500 year Return Period Event



**Pseudostatic Slope Stability Analysis
CCP Storage Facilities - Existing Conditions
Tennessee Valley Authority Fossil Plants**

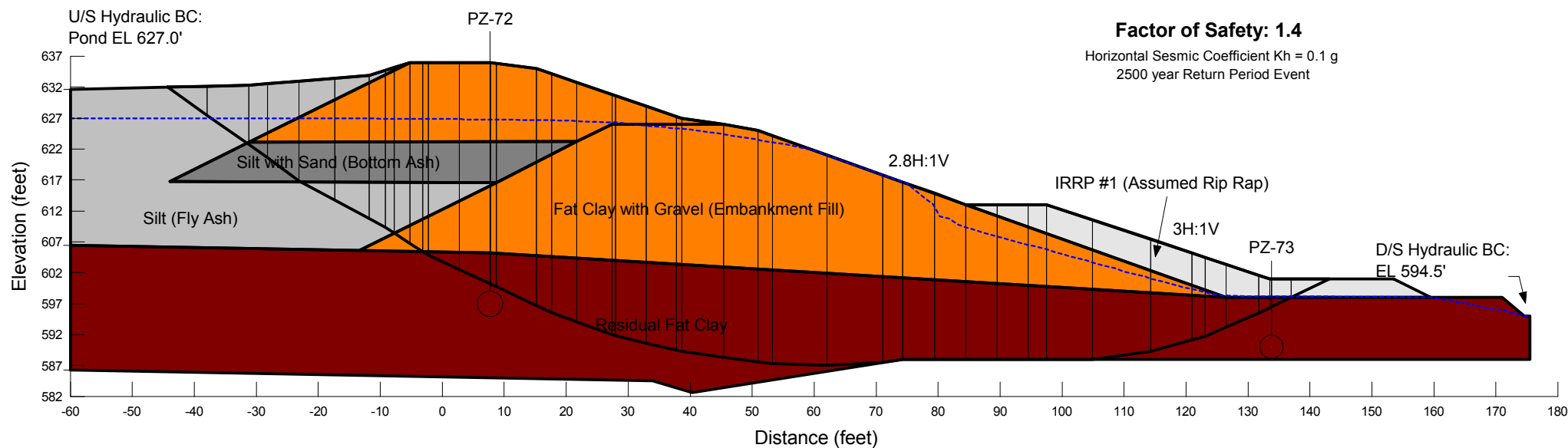
**Section J - Main Ash Pond
Widows Creek Fossil Plant
Stevenson, Alabama**



Note:
The results of analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. No warranties can be made regarding the continuity of subsurface conditions between the borings.

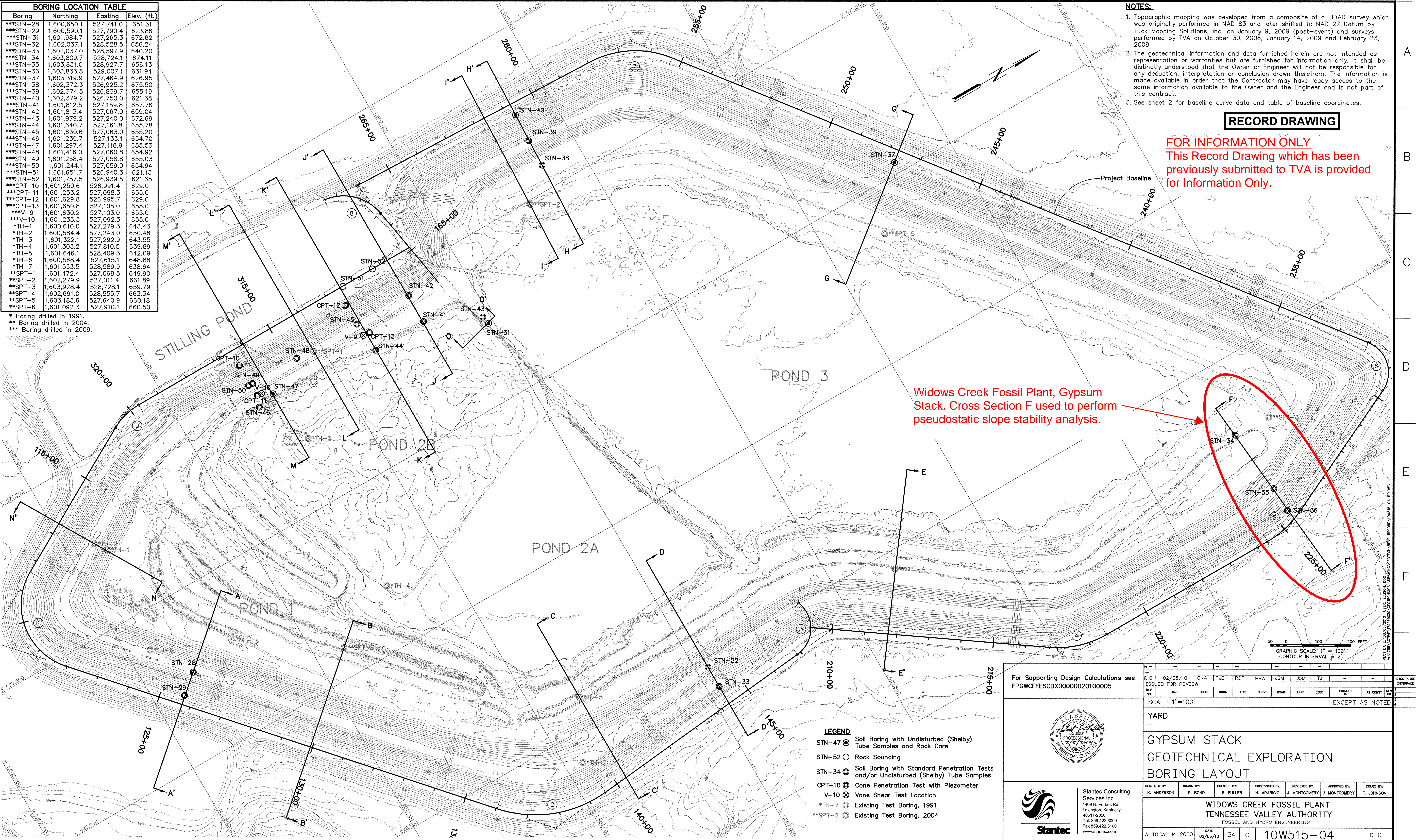
Additional remediation measures taken from URS plans dated 8/14/2010.

Material Type	Unit Weight	Cohesion	Friction Angle
Sand with Gravel (Bottom Ash)	119 pcf	0 psf	33 °
Rip Rap	115 pcf	0 psf	40 °
Silt (Fly Ash)	112 pcf	0 psf	21.8 °
Residual Fat Clay	125 pcf	650 psf	15.7 °
Fat Clay with Gravel (Embankment Fill)	125 pcf	1375 psf	14.2 °



BORING LOCATION TABLE			
Boring	Northing	Easting	Elev. (ft.)
***STN-28	1,600,650.1	527,741.0	651.31
***STN-29	1,600,590.1	527,790.4	623.86
***STN-31	1,601,984.7	527,265.3	672.82
***STN-32	1,602,037.1	528,528.5	656.24
***STN-33	1,602,037.0	528,597.9	640.20
***STN-34	1,603,809.7	528,724.1	674.11
***STN-35	1,603,831.0	528,927.7	656.13
***STN-36	1,603,833.8	529,007.1	631.94
***STN-37	1,603,319.9	527,464.9	626.95
***STN-38	1,602,372.3	526,925.2	675.50
***STN-39	1,602,374.5	526,839.7	655.19
***STN-40	1,602,379.2	526,750.0	621.38
***STN-41	1,601,812.5	527,159.8	657.76
***STN-42	1,601,813.4	527,067.0	659.04
***STN-43	1,601,979.2	527,240.0	672.89
***STN-44	1,601,640.7	527,161.8	655.78
***STN-45	1,601,630.6	527,063.0	655.20
***STN-46	1,601,239.7	527,133.1	654.70
***STN-47	1,601,297.4	527,118.9	655.53
***STN-48	1,601,416.0	527,060.8	654.92
***STN-49	1,601,258.4	527,058.8	655.03
***STN-50	1,601,244.1	527,059.0	654.94
***STN-51	1,601,651.7	526,940.3	621.13
***STN-52	1,601,757.5	526,939.5	621.65
***CPT-10	1,601,250.6	526,991.4	629.0
***CPT-11	1,601,253.2	527,098.3	655.0
***CPT-12	1,601,629.8	526,995.7	629.0
***CPT-13	1,601,650.8	527,105.0	655.0
***V-9	1,601,630.2	527,103.0	655.0
***V-10	1,601,235.3	527,092.3	655.0
*TH-1	1,600,610.0	527,279.3	643.43
*TH-2	1,600,584.4	527,243.0	650.48
*TH-3	1,601,322.1	527,292.9	643.55
*TH-4	1,601,303.2	527,810.5	639.89
*TH-5	1,601,646.1	528,409.3	642.09
*TH-6	1,600,568.4	527,615.1	648.88
*TH-7	1,601,553.5	528,589.9	638.64
**SPT-1	1,601,472.4	527,068.5	649.90
**SPT-2	1,602,279.9	527,011.4	661.89
**SPT-3	1,603,928.4	528,728.1	659.79
**SPT-4	1,602,691.0	528,555.7	663.34
**SPT-5	1,603,183.6	527,640.9	660.18
**SPT-6	1,601,092.3	527,910.1	660.50

* Boring drilled in 1991.
** Boring drilled in 2004.
*** Boring drilled in 2009.



NOTES:

- Topographic mapping was developed from a composite of a LiDAR survey which was originally performed in NAD 83 and later shifted to NAD 27 Datum by Tuck Mapping Solutions, Inc. on January 9, 2009 (post-event) and surveys performed by TVA on October 30, 2006, January 14, 2009 and February 23, 2009.
- The geotechnical information and data furnished herein are not intended as representation or warranties but are furnished for information only. It shall be distinctly understood that the Owner or Engineer will not be responsible for any deduction, interpretation or conclusion drawn therefrom. The information is made available in order that the Contractor may have ready access to the same information available to the Owner and the Engineer and is not part of this contract.
- See sheet 2 for baseline curve data and table of baseline coordinates.

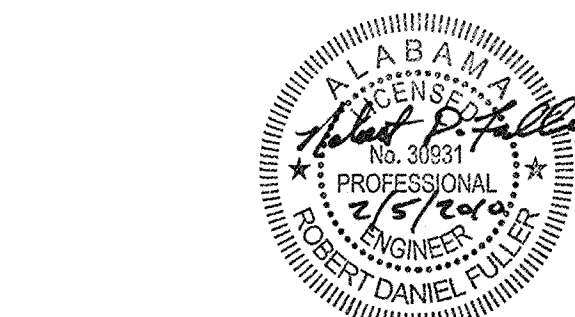
RECORD DRAWING

FOR INFORMATION ONLY
This Record Drawing which has been previously submitted to TVA is provided for Information Only.

Widows Creek Fossil Plant, Gypsum Stack. Cross Section F used to perform pseudostatic slope stability analysis.

LEGEND

- STN-47 Soil Boring with Undisturbed (Shelby) Tube Samples and Rock Core
- STN-52 Rock Sounding
- STN-34 Soil Boring with Standard Penetration Tests and/or Undisturbed (Shelby) Tube Samples
- CPT-10 Cone Penetration Test with Piezometer
- V-10 Vane Shear Test Location
- *TH-7 Existing Test Boring, 1991
- **SPT-3 Existing Test Boring, 2004



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For Supporting Design Calculations see
FPGWCFFESCDX00000020100005

R 0 02/05/10 GKA PJB RDF HRA JSM JSM TJ - - -										DISCIPLINE
ISSUED FOR REVIEW										INTERFACE
REV	NO.	DATE	DSGN	DRWN	CHKD	SUPV	RVMD	APPD	ISSD	PROJECT
SCALE: 1"=100'										EXCEPT AS NOTED
YARD										
GYPSUM STACK										
GEOTECHNICAL EXPLORATION										
BORING LAYOUT										
DESIGNED BY:	DRAWN BY:	CHECKED BY:	SUPERVISED BY:	REVIEWED BY:	APPROVED BY:	ISSUED BY:				
K. ANDERSON	P. BOND	R. FULLER	H. APARICIO	J. MONTGOMERY	J. MONTGOMERY	T. JOHNSON				
WIDOWS CREEK FOSSIL PLANT										
TENNESSEE VALLEY AUTHORITY										
FOSSIL AND HYDRO ENGINEERING										
AUTOCAD R 2000	DATE	34	C	10W515-04	R 0					
02/05/10										

